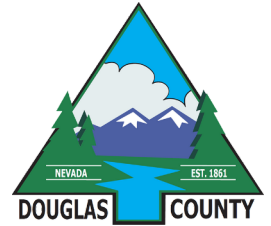


Plan Check Comments

Douglas County Community Development

1594 Esmeralda Ave
Minden, NV 89423



Permit Type: Commercial Permit

Project Description: New Storage building for DCSD (Pinon Hills)

Document Name: Plans SUB 2

Report Date: 11.30.2022

Application Number: DB22-1491

Site Address:
1476 STEPHANIE WY
Minden, NV 89423

Reviewer Contact Information:

Reviewer Name	Reviewer Email	Reviewer Phone No.:
Rebecca Spates	rspates@douglasnv.us	775-782-6238
Lucille Rao	lrao@douglasnv.us	1
Tim Davis	tdavis@douglasnv.us	775-782-6224
Natalia Morelli	NMorelli@douglasnv.us	(775)783-6421

General Comments

Corrections in the following table need to be applied before a permit can be issued

Division - Reviewer	Page Ref	Comments
Engineering - Rebecca Spates	1	Submittal 2

Douglas County Community Development Johnson Lane Park Storage Building

Douglas County
 Planning Division
APPROVED
 DB22-1491 LR

All utilities, including power, must
 be underground, unless waived by
 Douglas County, per DCC 20.220.050

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 BUILDING DIVISION
 BUILDER AND OWNER
 RESPONSIBLE FOR COMPLIANCE
 WITH ALL APPLICABLE CODES
 ALL WORK SUBJECT TO FIELD
 INSPECTION APPROVAL

project

Douglas County Community Development
 Johnson Lane Park Storage Building
 Douglas County Community Development
 1594 Esmeralda Avenue
 Minden, Nevada 89423

revisions

No.	Description	Date

drawn by MLM
 reviewed by PAC
 date 11/10/2022
 project number 22018
 drawing name

Cover Sheet

sheet number

A000

Douglas County Community Development
 1594 Esmeralda Avenue
 Minden, Nevada 89423

APPROVED
 Douglas County
 Community Development
 BUILDING DIVISION

DATE 11.30.2022 PERMIT # DB22-1491
 BY

The Approval of Plans and Specifications does not
 Permit the violation of any Section of the Building
 Code or any County Ordinance or State Law.

THIS SET OF PLANS MUST BE
 KEPT ON BUILDING SITE AT ALL
 TIMES DURING PROGRESS OF
 CONSTRUCTION FOR INSPECTION
 PURPOSES. SEC. 106.3.1

100% Construction Documents
 11/10/2022



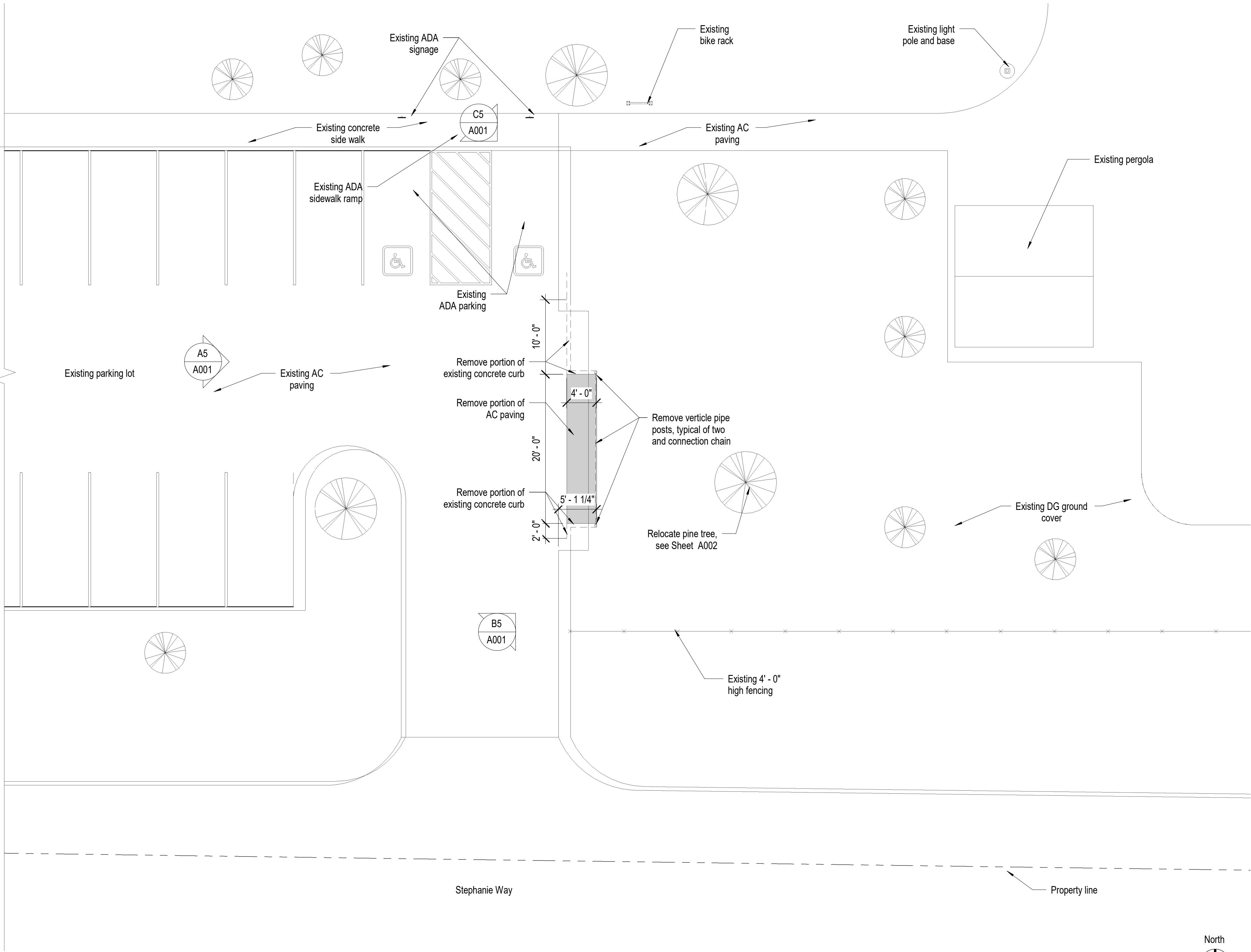
A5 Photo of Existing Conditions



B5 Photo of Existing Conditions



C5 Photo of Existing Conditions



D4 Architectural Partial Demolition Site Plan
1/8" = 1'-0"

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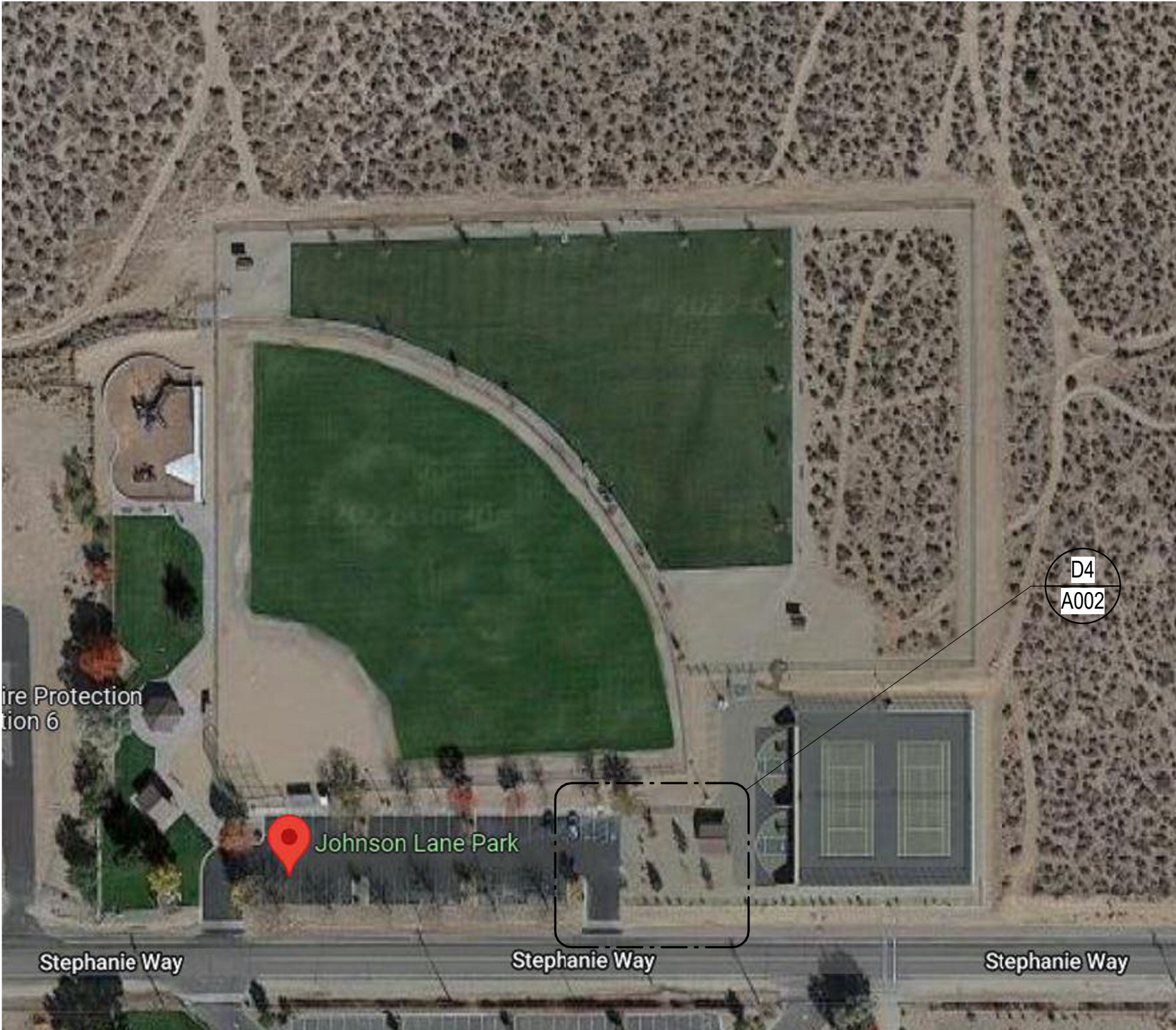
No.	Description	Date

drawn by	Author
reviewed by	PAC
date	11/10/2022
project number	22018
drawing name	

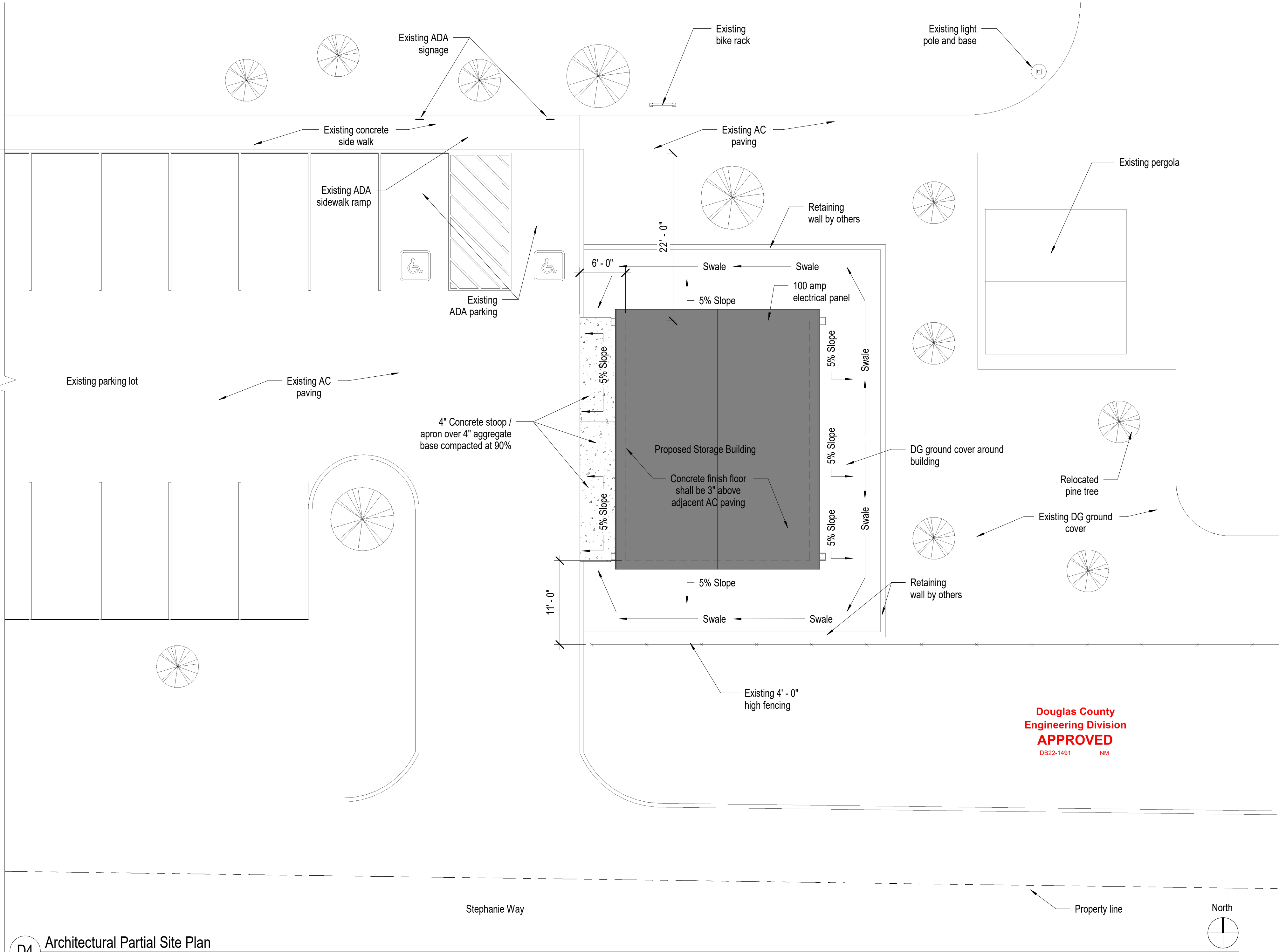
**Architectural
Partial Demolition
Site Plan**

sheet number

A001



B5 Aerial View



D4 Architectural Partial Site Plan
1/8" = 1'-0"

Site Plan Notes

1. Coordinate construction operations, schedule, and sequencing with Douglas County Community Development Project Manager.
2. Contractor is responsible for safety and security of the Contractor parking and staging area. Temporary fencing is recommended for securing construction materials, vehicles, equipment, etc. at the staging area.
3. The Contractor parking and staging area must maintain a clear path for emergency vehicles to circulate around the building at all times.
4. At the conclusion of the project the Contractor shall clean the site area and restore to original appearance, including but not limited to replacement of the existing AC paving if damaged beyond original condition.
5. All cross slopes on the accessible rout shall be less than 2%, all running slopes shall be 5% maximum.

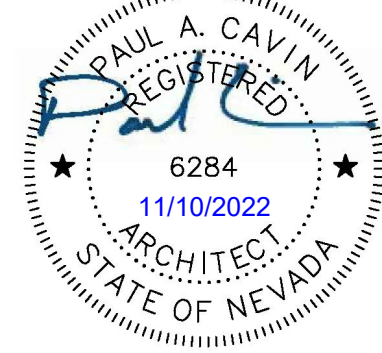
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Douglas County Community Development
1594 Esmeralda Avenue
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revisions

No.	Description	Date

drawn by	MLM
reviewed by	PAC
date	11/10/2022
project number	22018
drawing name	

Architectural
Partial Site Plan

sheet number

A002

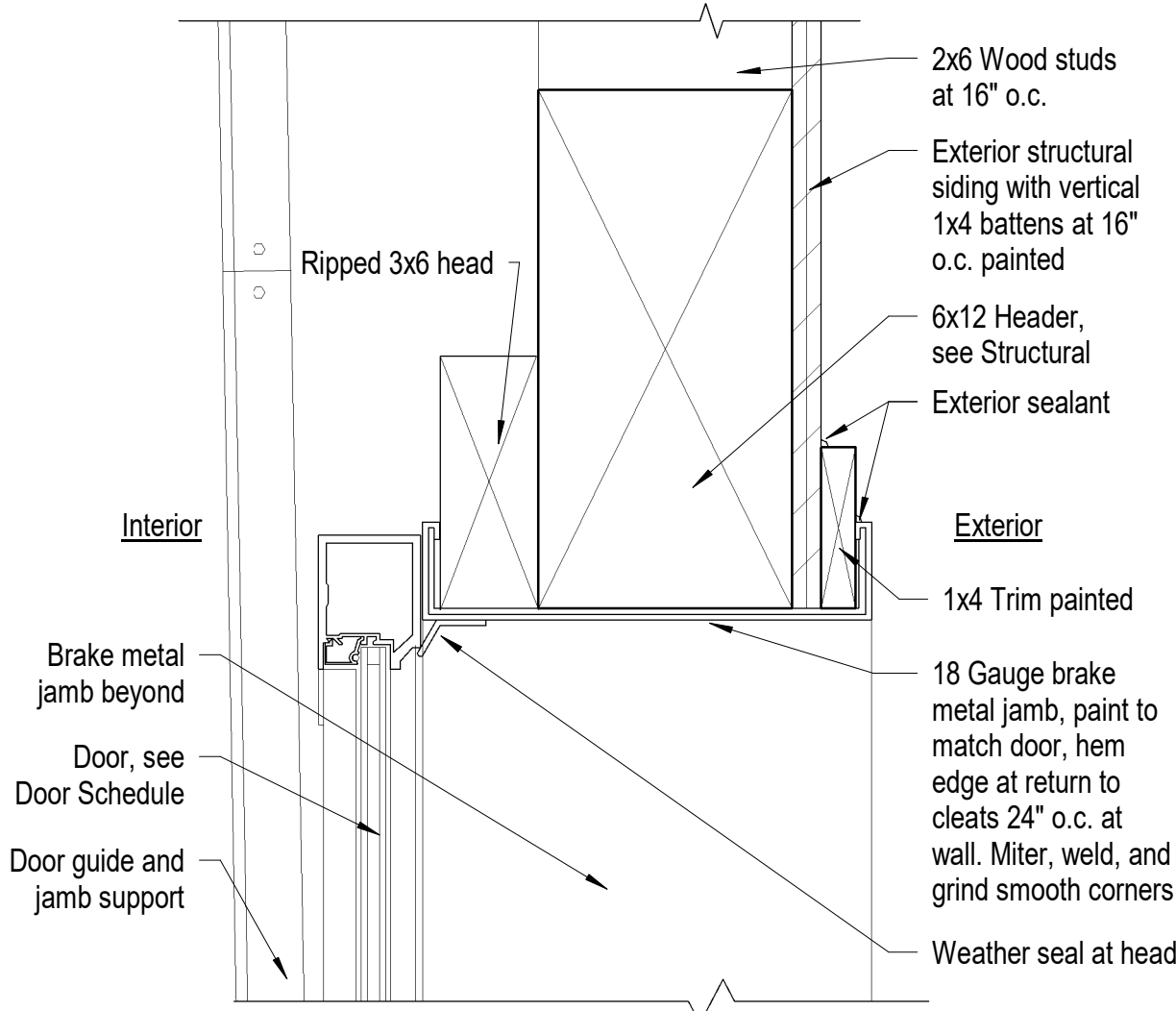
Door Schedule																
Door Number	Door					Frame				Details				Label	Hardware Group	Remarks
	Size (width x height)	Pair	Material	Type	Glass	Material	Size	Type	Glass	Head	Strike	Hinge	Sill			
100B	10' - 0" x 10' - 0"		ST	SO		ST				B5	C4	C4				
100A	10' - 0" x 10' - 0"		ST	SO		ST				B5	C4	C4				
100C	3' - 0" x 7' - 0"		HM	F		HM	7-3/4"	1		B4	C4	C4	D4		H1	

Door Notes

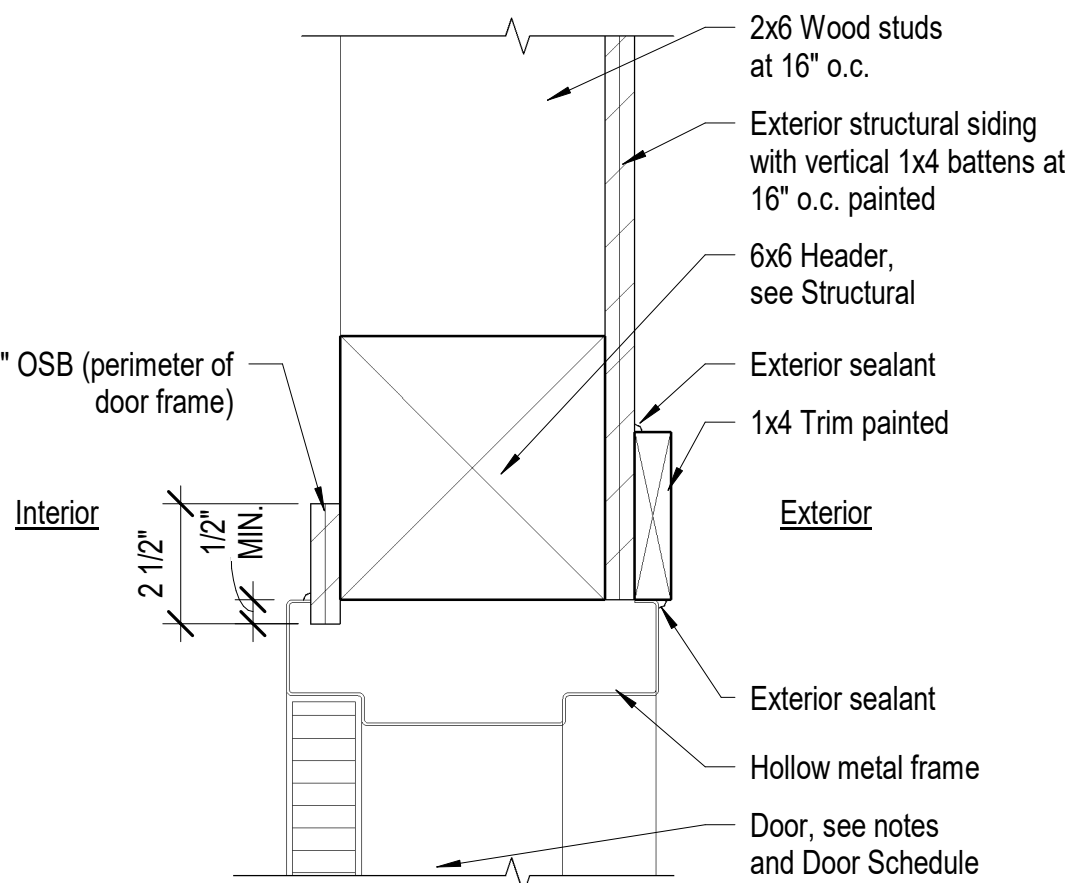
- See Specifications for additional information and requirements.
- The hardware supplier shall coordinate cylinders and keying with the client building staff.
- All new doors and hardware shall meet current ADA and ANSI standards.

Door Schedule Legend

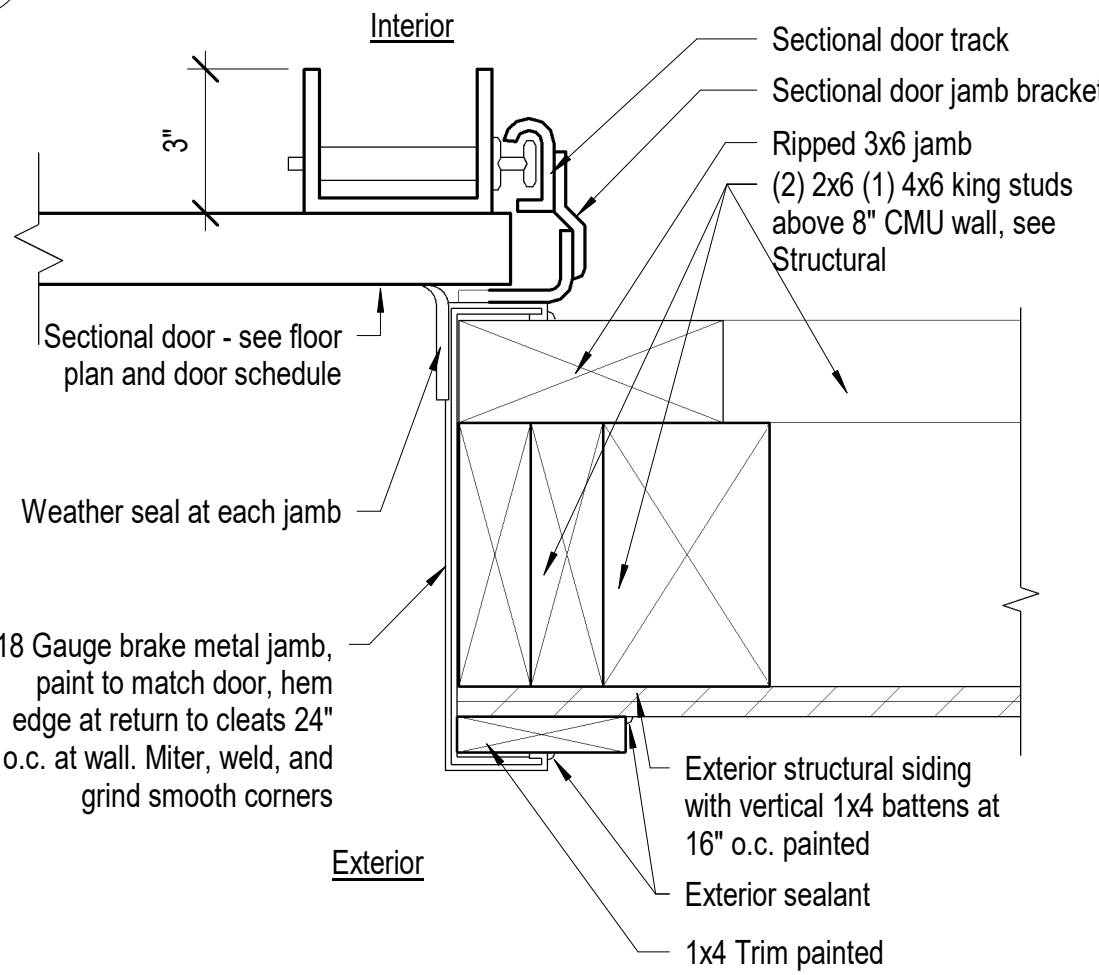
- ST - Steel
- HM - Hollow metal
- V.I.F. - Verify in field



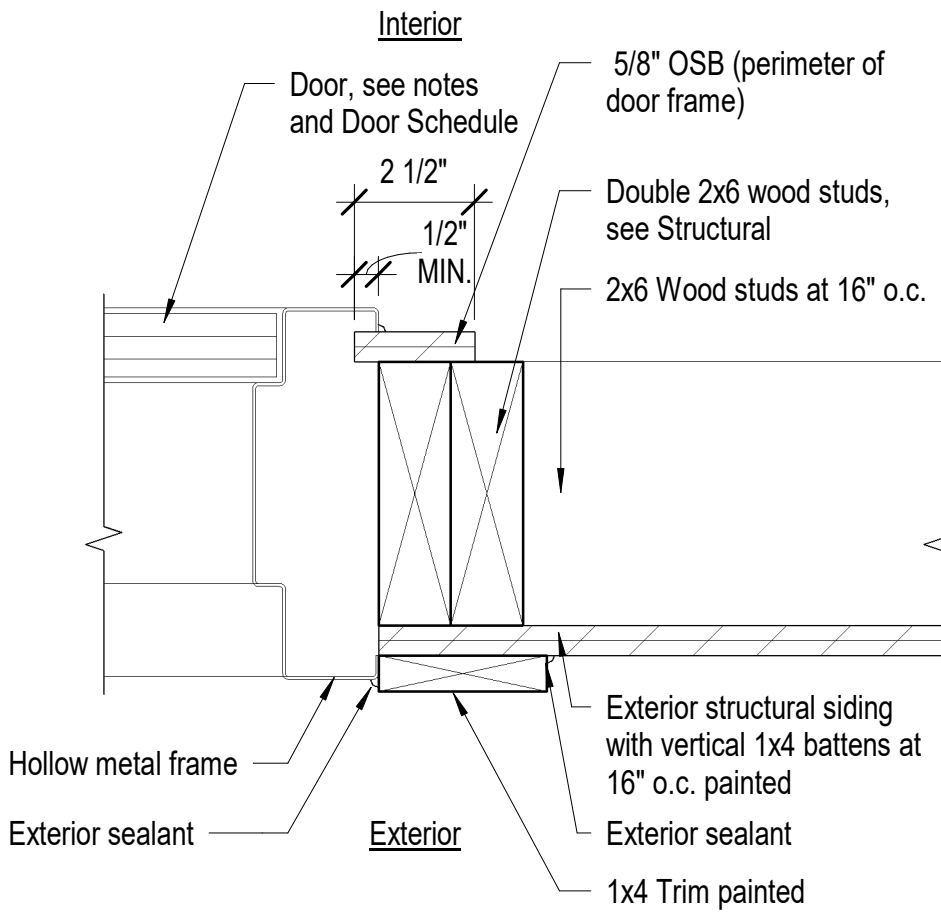
B5 Sectional Door Head Detail
3" = 1'-0"



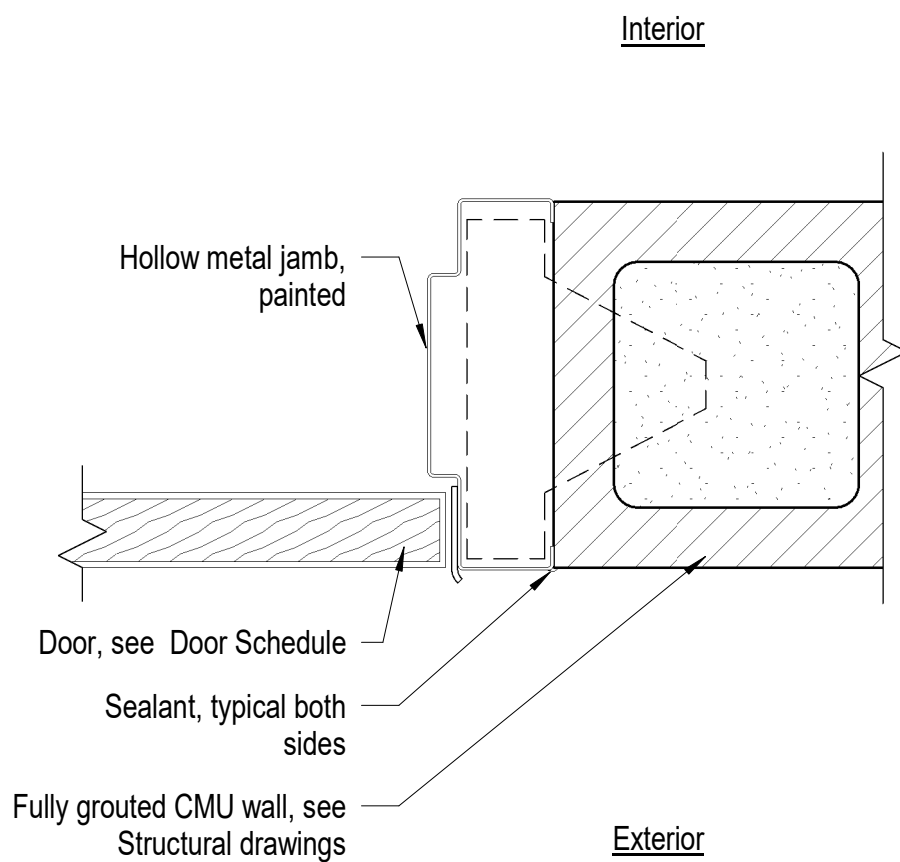
B4 HM Head Detail
3" = 1'-0"



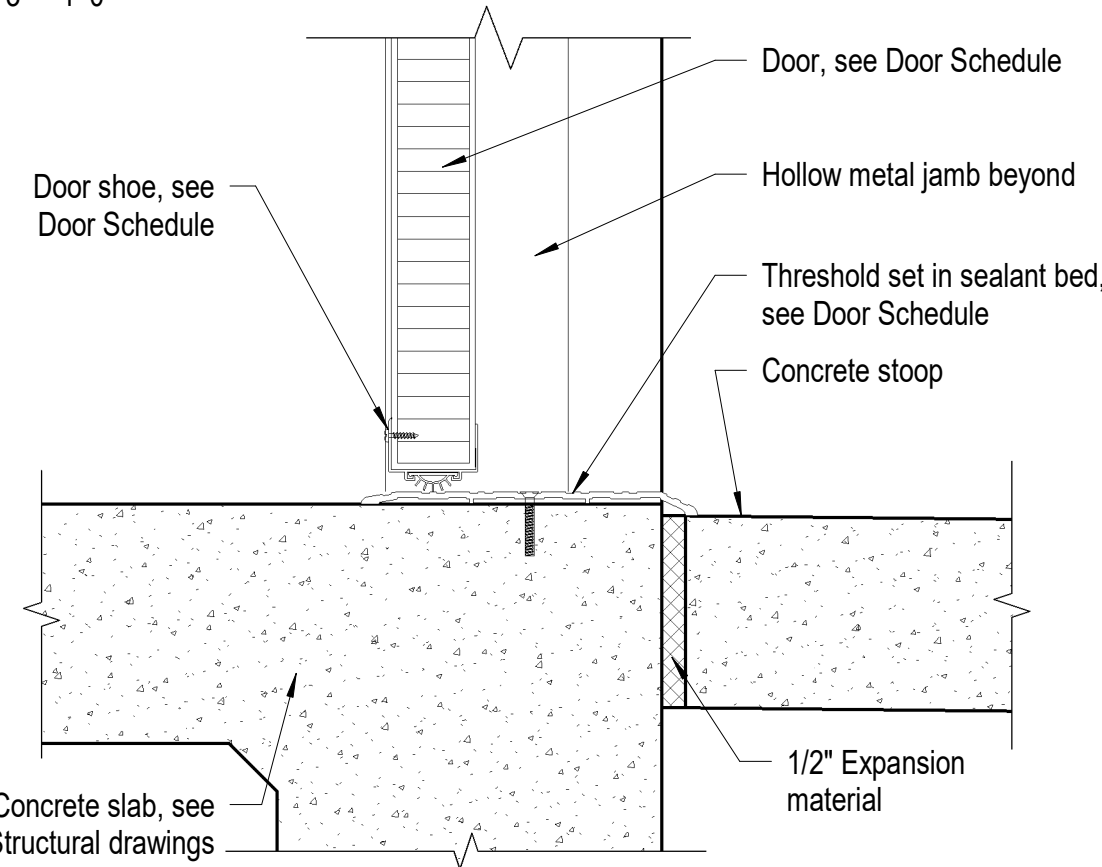
C5 Sectional Door Jamb Detail
3" = 1'-0"



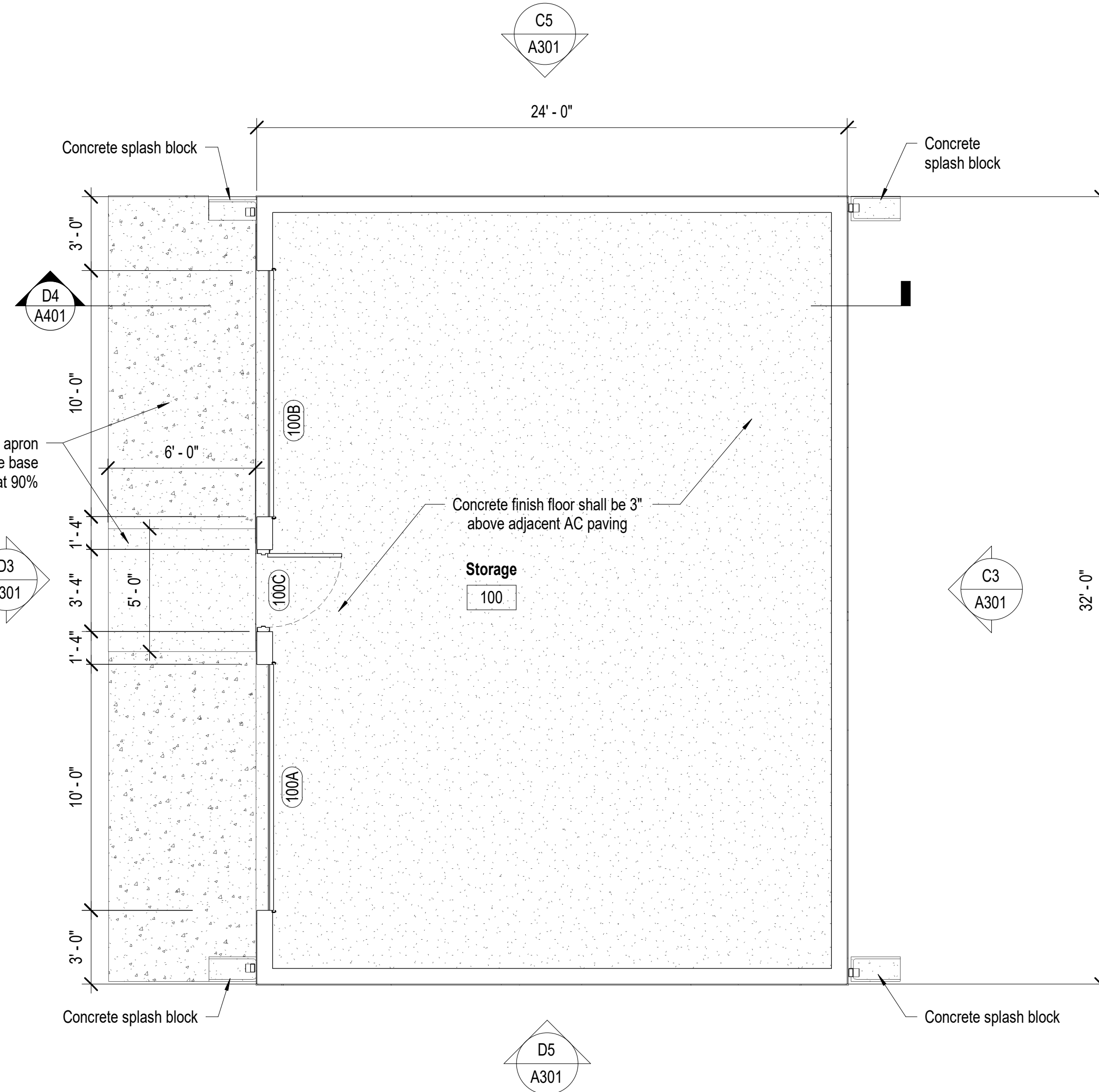
C4 HM Jamb at Stud Wall Detail
3" = 1'-0"



1 HM Jamb at CMU Wall Detail
3" = 1'-0"



D4 HM Threshold Detail
3" = 1'-0"



D3 Floor Plan
1/4" = 1'-0"

Floor Plan Notes

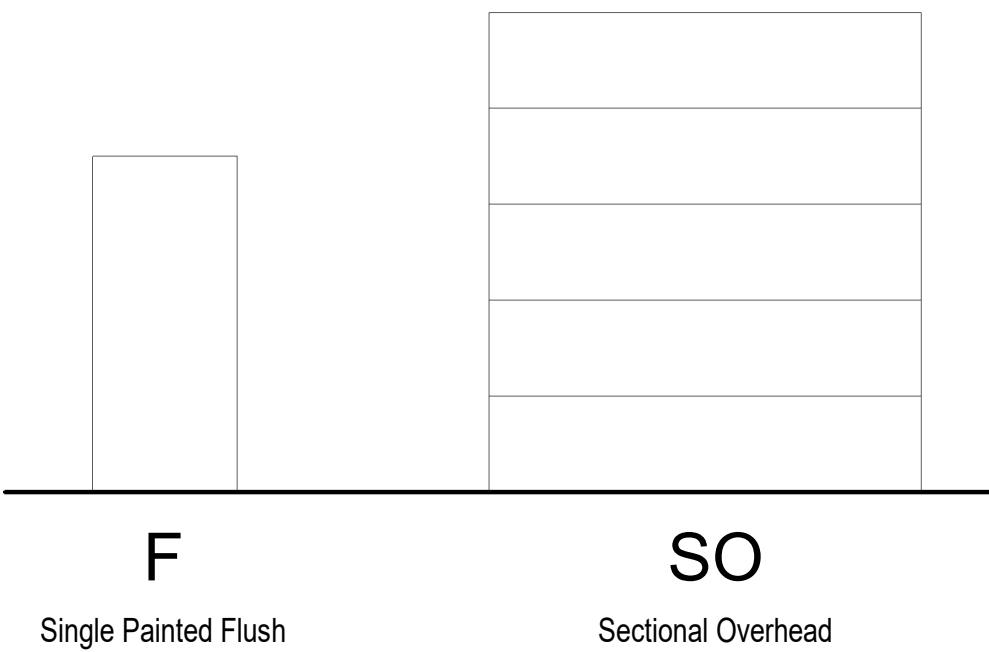
- Coordinate construction operations, schedule, and sequencing with Douglas County Community Development Project Manager.
- All dimensions are from face of stud to face of stud, unless noted otherwise.
- The Contractor will be responsible for setting the exact limits of construction required in order to perform the work.
- The Contractor shall maintain a clean environment during all constructions operations, and shall conduct a final cleaning of entire area of work at the conclusion of the project.
- The Contractor shall protect existing finishes from construction traffic, cutting, and all construction activities.
- See Structural drawings and Specifications for additional information and requirements.

Door Hardware Groups

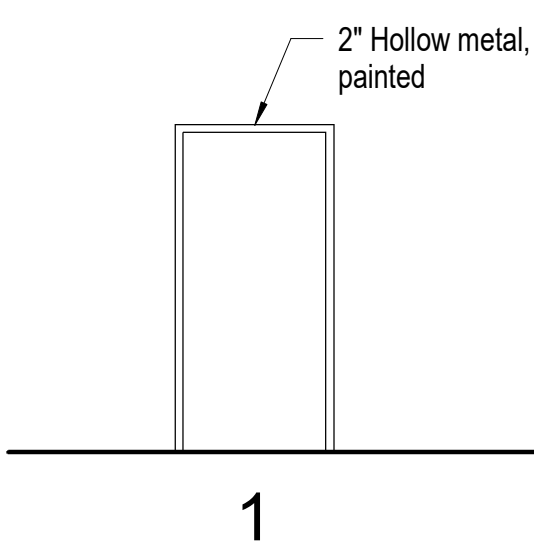
- Door Hardware groups are as follows:

H1	Quantity	Item	Description	Manufacturer	Finish
3	Hinges	BB1279		Hager	US26D
1	Lever Set	ND60PD RHO		Schlage	US26D
1	Dead Bolt	B562		Schlage	US26D
1	Closer	4040XP		LCN	Aluminum
1	Door Shoe	217PK		Pemko	Aluminum
1	Door Seals	S88BL		Pemko	Black
1	Threshold (ADA)	271		Pemko	Aluminum
- All locksets to use cylindrical type cylinders.
- Keying shall be coordinated with Douglas County Community Department Project Manager.
- All exterior thresholds to be set in full bed of sealant.

Door Types



Frame Type



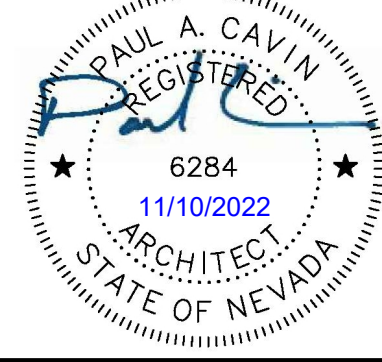
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INSPECTION APPROVAL

project

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Johnson Lane Park Storage Building
Douglas County Community Development
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revisions

No.	Description	Date

drawn by MLM
reviewed by PAC
date 11/10/2022
project number 22018
drawing name

Floor Plan

sheet number

A101

Roof Plan Notes

1. Coordinate construction operations, schedule, and sequencing with Douglas County Community Development Project Manager.
2. The roof system is as follows:
 - A. Composition shingle roofing over
 - B. #30 Felt over
 - C. Roof sheathing over (see Structural drawings)
 - D. Trusses at 24" o.c.
3. In the event that roof work creates a condition where interior spaces are open to weather, the Contractor shall protect the building from the effects of exposure to exterior conditions. The building shall be weather-tight at the conclusion of work each day. At the conclusion of work in a specific area the Contractor is to replace all removed components to a weather-tight condition. In the event that the roof leaks, the Contractor shall respond and make the roofing water tight within 3 hours, at no additional cost to the Owner.
4. The Contractor shall maintain a clean environment during all constructions operations, and shall conduct a final cleaning of entire area of work at the conclusion of the project.
5. See Structural drawings and specifications for additional information and requirements.
6. Where roofing and flashing requirements vary from those described on the details, Contractor shall provide roofing manufacturer's approved details as required for warranty requirements.
7. All penetrations, flashings, and roofing components are to be installed per roof manufacturer's instructions and recommendations as required for warranty.
8. Contractor shall completely protect the entire roof for the duration of construction procedures on the roof. Protection shall include covering necessary to protect the roof from foot traffic, equipment, weather, rain, and other potentially damaging sources.

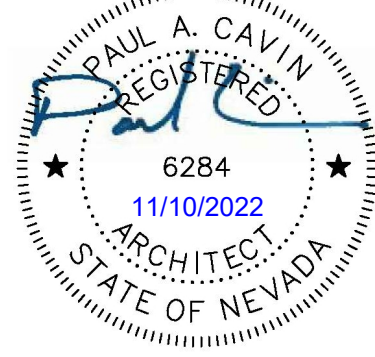
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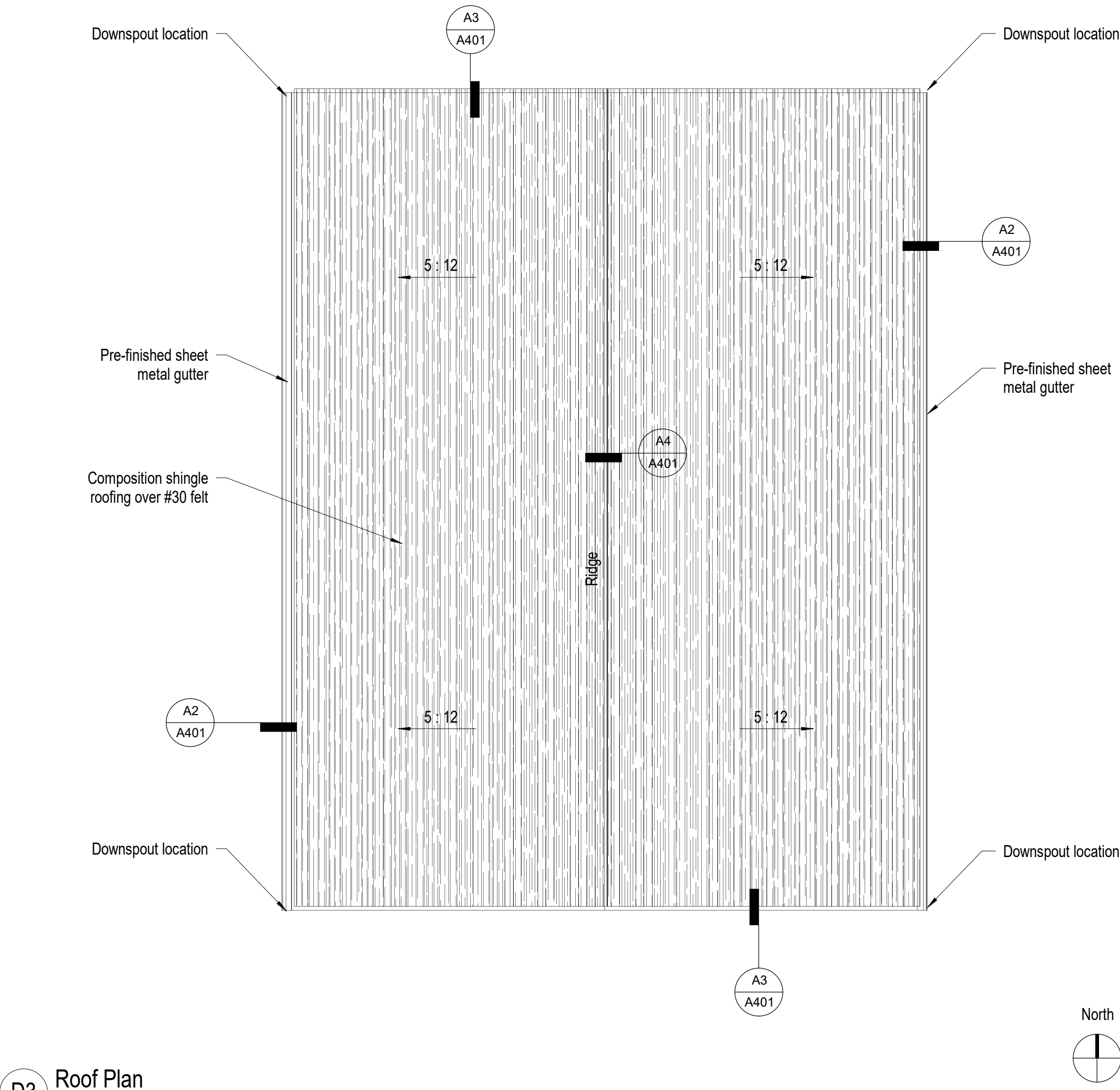
No.	Description	Date

drawn by MLM
reviewed by PAC
date 11/10/2022
project number 22018
drawing name

Roof Plan

sheet number

A201



D3 Roof Plan
1/4" = 1'-0"

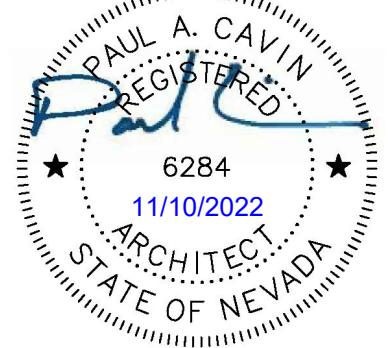
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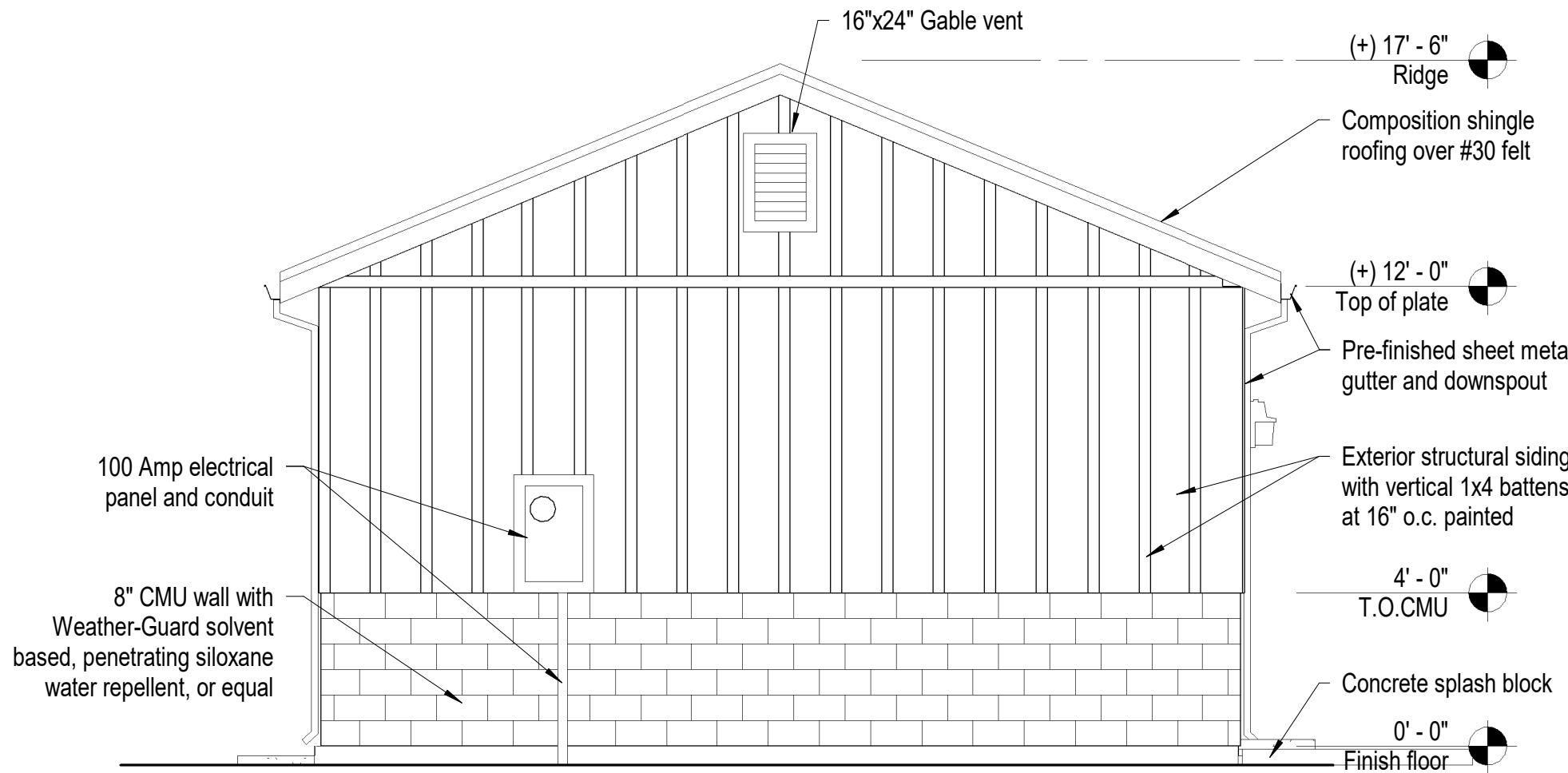
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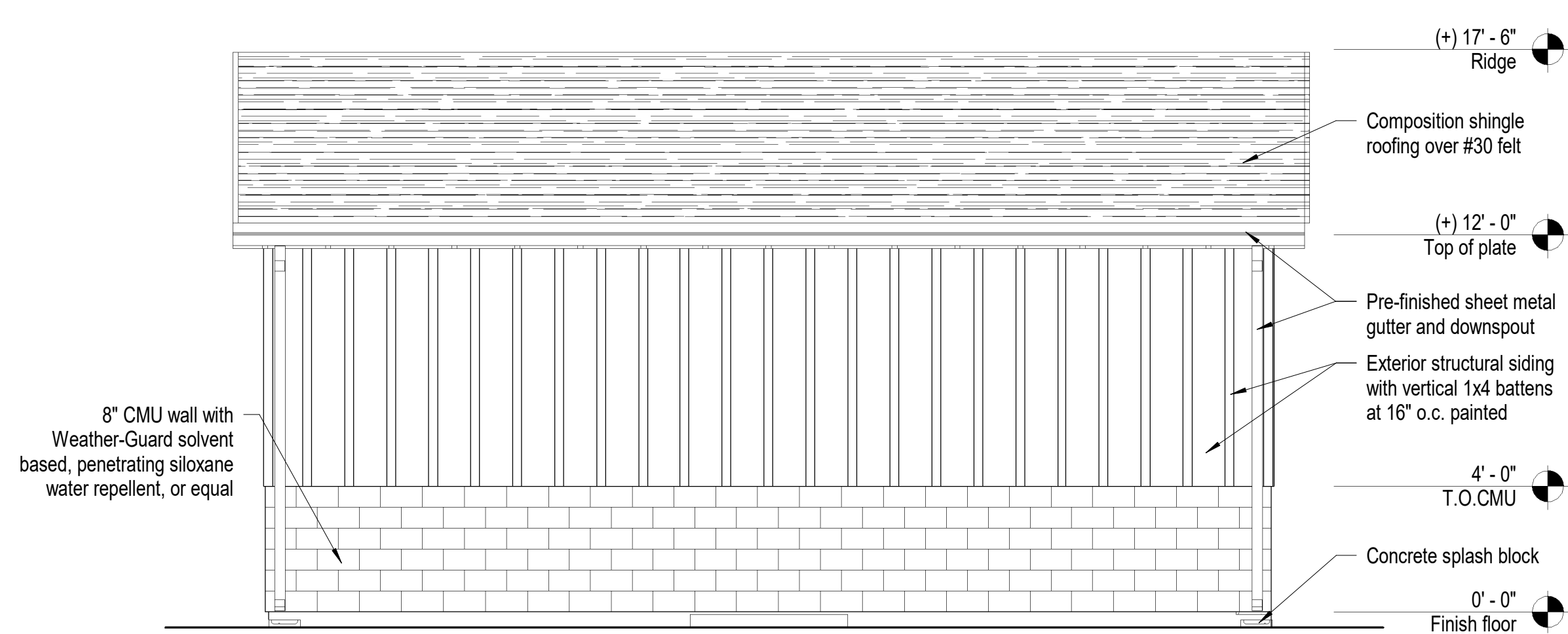
Exterior Elevations

sheet number

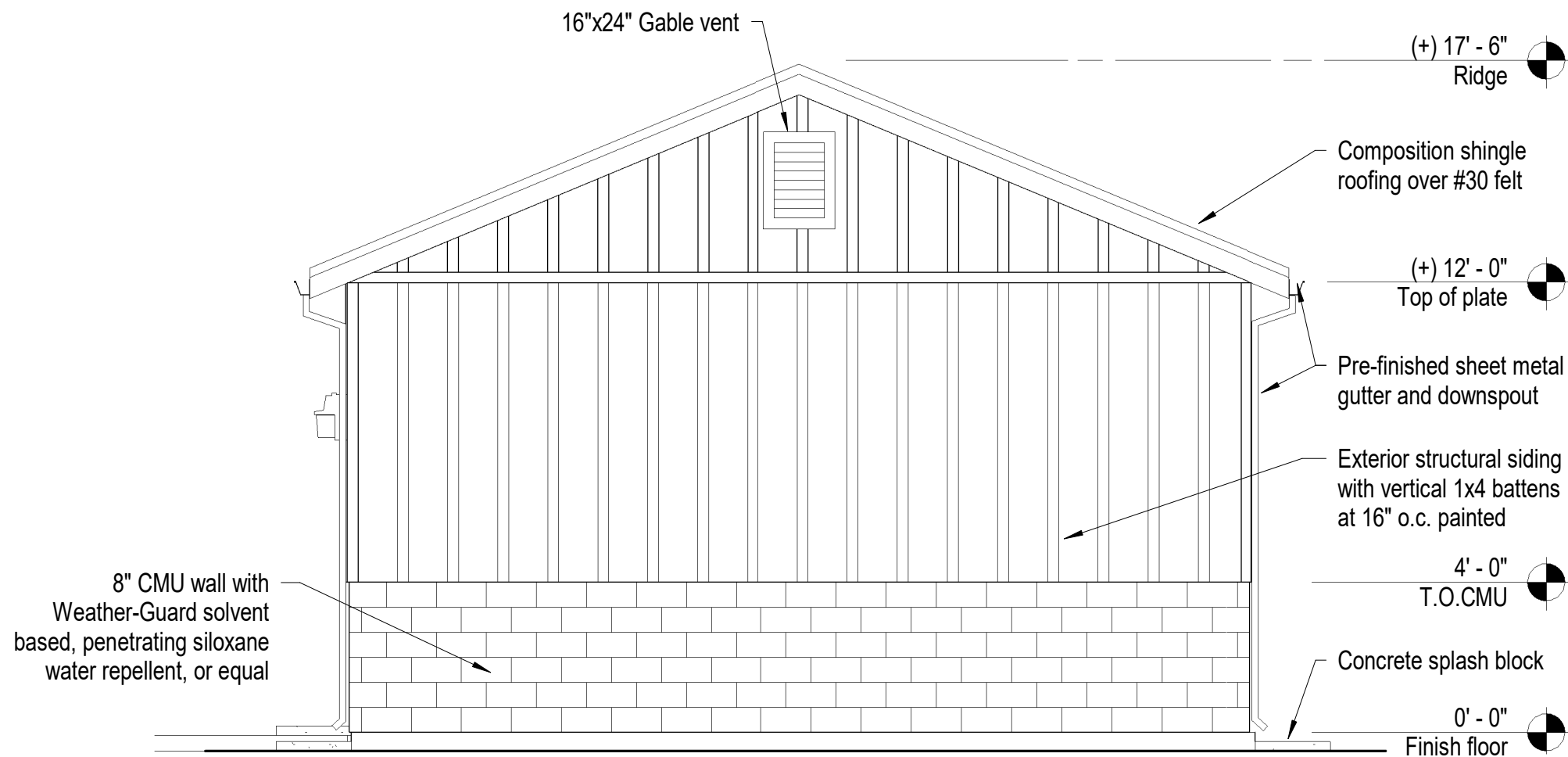
A301



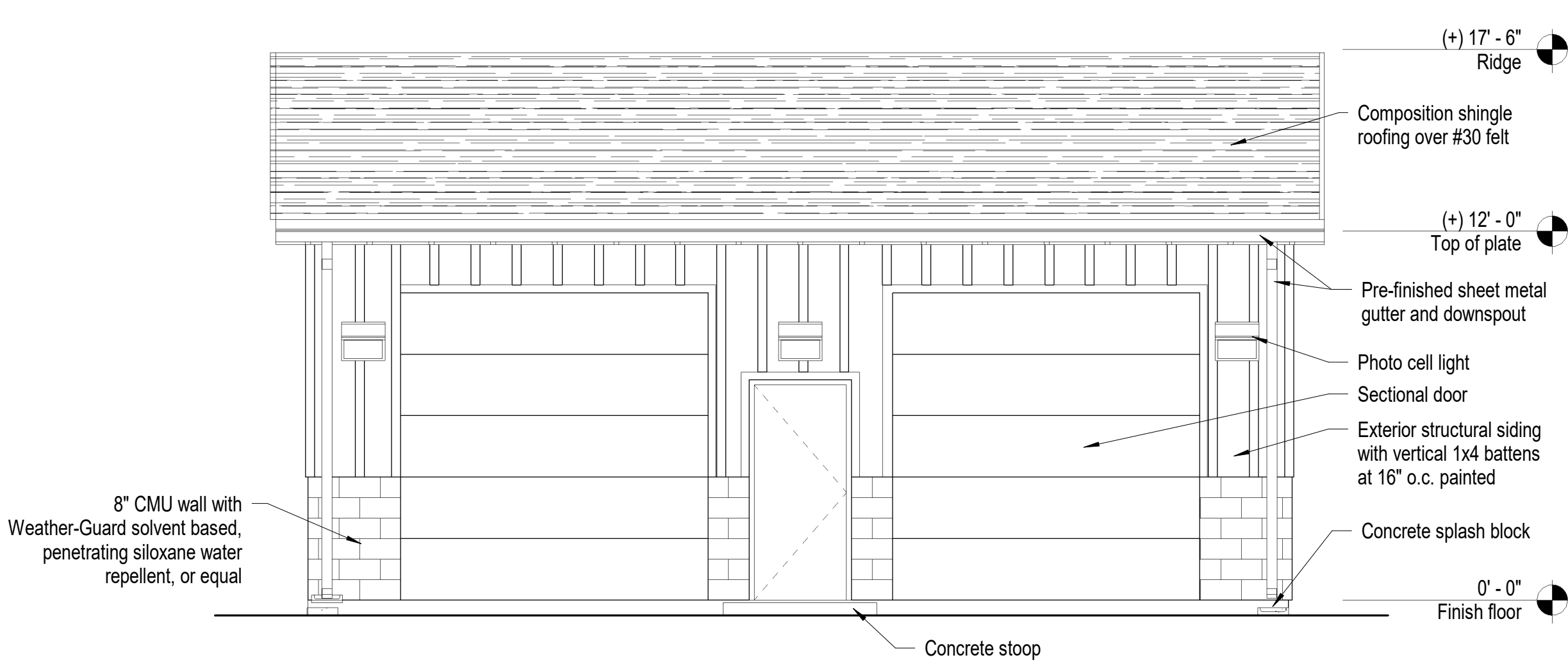
C5 North Elevation
1/4" = 1'-0"



C3 East Elevation
1/4" = 1'-0"



D5 South Elevation
1/4" = 1'-0"



D3 West Elevation
1/4" = 1'-0"

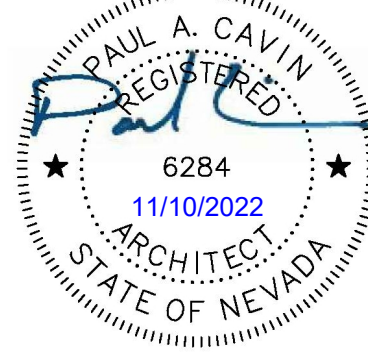
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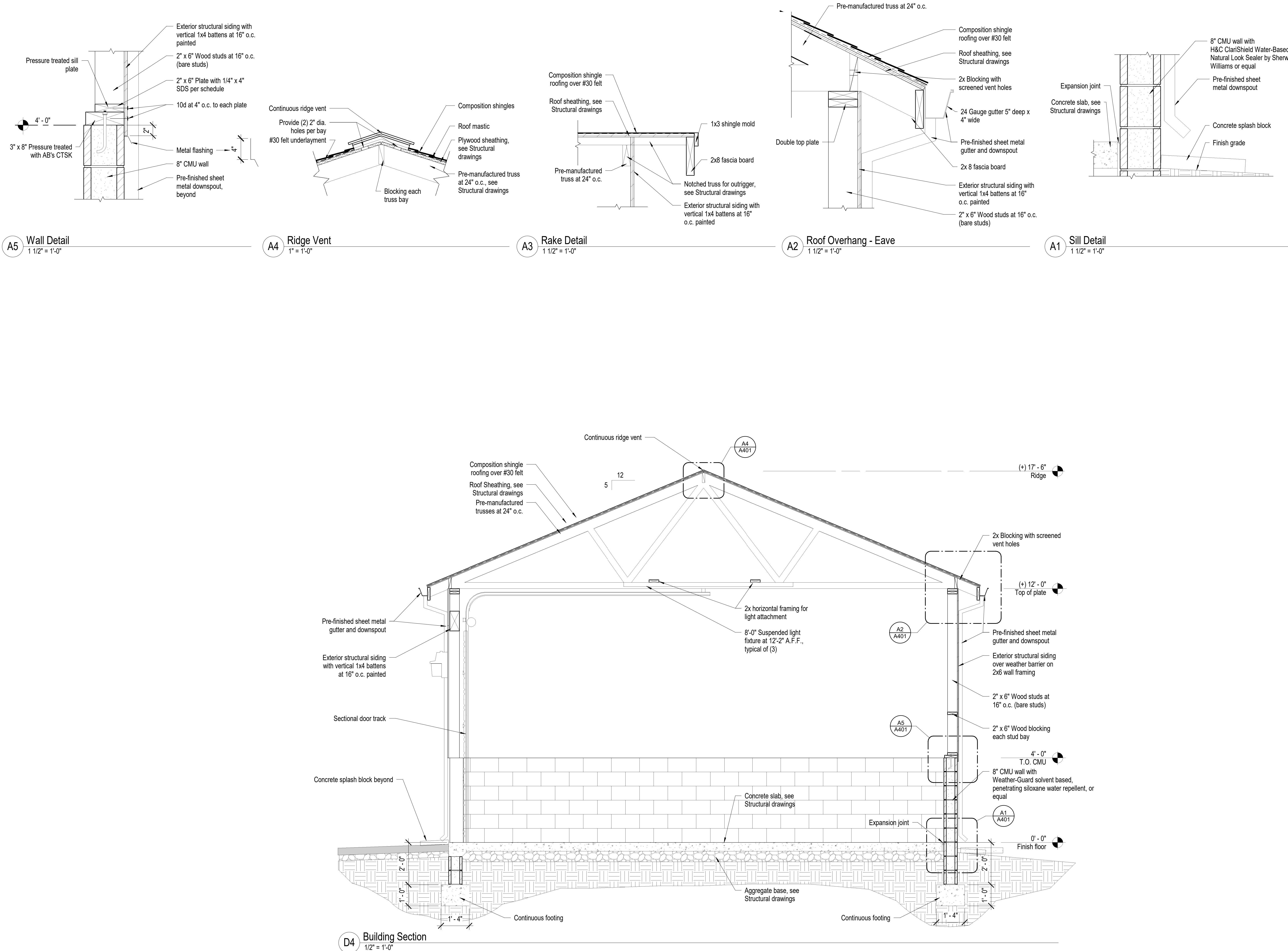
No.	Description	Date

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reviewed by PAC
date 11/10/2022
project number 22018
drawing name

Building Sections

sheet number

A401



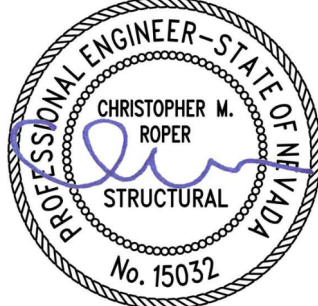
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EXP: 12/31/2023 11/10/2022

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CFBR STRUCTURAL INC
5425 LOUIE LANE, RENO, NV

project

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Minden, Nevada 89423

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reviewed by CMR
date 11/10/2022
project number 22018
drawing name

STRUCTURAL COVER SHEET & NOTES

sheet number

S0.1

STRUCTURAL DESIGN CRITERIA

- CODE: 2018 INTERNATIONAL BUILDING CODE (IBC)
STRUCTURAL RISK CATEGORY II (NORMAL OCCUPANCY)
- SNOW LOADS:
GROUND SNOW LOAD, Pg: 40 PSF
Cs: 1.0
Cd: 1.2
Is: 1.0
ROOF SNOW LOAD, S: 34 PSF
- WIND LOADS:
BASIC WIND SPEED, V: 120 MPH
EXPOSURE CATEGORY: C
ENCLOSURE CLASSIFICATION: ENCLOSED (Gc) = 0.18)
- SEISMIC LOADS:
Ss: 0.25
S1: 0.65
SITE CLASS: D
Sds: 1.40
Sd1: 0.74
Is: 1.00
SEISMIC DESIGN CATEGORY: D
SEISMIC FORCE RESISTING SYSTEM(S): WOOD SHEARWALLS (R=6.5)
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
BASE SHEAR, V=CsW: 0.23W
- SOILS:
IBC TABLE 1806.2: CLASS 4 MATERIALS ASSUMED
ALLOWABLE SOIL BEARING PRESSURE: 1500 PSF
COEFFICIENT OF FRICTION: 0.25
PASSIVE PRESSURE: 250 PCF
FROST DEPTH: 24"
- GENERAL
- A. THESE GENERAL NOTES APPLY TO ALL WORK SHOWN IN THE STRUCTURAL DRAWINGS. THE GENERAL CONTRACTOR AND ALL INVOLVED PARTIES SHALL BE DIRECTLY RESPONSIBLE FOR READING AND COMPLYING WITH ALL INFORMATION PROVIDED IN THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
- B. THE CONTRACTOR SHALL COMPLY WITH ALL GENERAL NOTES AND TYPICAL DETAILS SHOWN IN THE DRAWINGS, WHETHER OR NOT SPECIFIC FLAGGING OR REFERENCE HAS BEEN MADE TO THE APPLICABLE GENERAL NOTE OR TYPICAL DETAIL. PROJECT SPECIFIC NOTES AND DETAILS SHOWN ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- C. ALL WORK SHALL COMPLY WITH THE MINIMUM STANDARDS OF THE CURRENT ADOPTED BUILDING CODE, THE LATEST EDITION OF ASTM OR OTHER INDUSTRY STANDARDS REFERENCED, AND ALL APPLICABLE LOCAL, STATE AND FEDERAL REQUIREMENTS AND REGULATIONS. FOR ITEMS, METHODS, AND/OR MATERIALS NOT SHOWN, ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE REGULATING AGENCIES THAT HAVE AUTHORITY OVER SUCH PORTIONS OF WORK.
- D. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKERS, AND OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, PREPARATION AND EXECUTION OF A SAFETY PROGRAM AND DESIGN AND INSTALLATION OF BRACING, SHORING, FORMS AND SCAFFOLDING. THE CONTRACTOR SHALL RETAIN HIS OWN ENGINEER WHERE REQUIRED FOR MEANS AND METHODS AS WELL AS ANY OTHER DELEGATED DESIGN ITEMS.
- E. THE CONTRACTOR SHALL BE DIRECTLY RESPONSIBLE FOR VERIFYING ALL DIMENSIONS, ELEVATIONS, GRADES AND CONDITIONS WITH ARCHITECTURAL AND OTHER DRAWINGS (I.E. CIVIL, MECHANICAL, PLUMBING, ELECTRICAL, ETC.) PRIOR TO CONSTRUCTION. THE ARCHITECT AND ENGINEER (A/E) SHALL BE NOTIFIED OF ANY DISCREPANCIES, OMISSIONS OR INCONSISTENCIES SO REVISIONS OR CLARIFICATIONS CAN BE MADE WHERE NECESSARY.
- F. IN THE EVENT EXISTING CONDITIONS ARE FOUND TO BE DIFFERENT FROM THOSE SHOWN IN THE DRAWINGS, THE A/E SHALL BE NOTIFIED SO REVISIONS OR CLARIFICATIONS CAN BE MADE WHERE NECESSARY.
- G. NO CHANGES OR DEVIATIONS FROM THE PLANS AND SPECIFICATIONS WILL BE ALLOWED WITHOUT WRITTEN AUTHORIZATION FROM THE A/E. ANY DESIRED CHANGES OR DEVIATIONS SHALL BE PRESENTED TO THE A/E FOR REVIEW WITH NO GUARANTEE THAT THE SUBSTITUTION WILL BE ALLOWED.
- H. DO NOT SCALE THE DRAWINGS. DRAWINGS ARE DIAGRAMMATIC AND MAY NOT SCALE ACCURATELY. STRUCTURAL DRAWINGS AND GRAPHICS ARE INTENDED TO BE VIEWED AND PRINTED IN COLOR. PRINTING OR VIEWING IN BLACK-AND-WHITE IS NOT RECOMMENDED.
- I. THE CONTRACTOR SHALL BE DIRECTLY RESPONSIBLE FOR COORDINATING THE FOLLOWING TYPES OF ITEMS WHICH ARE TYPICALLY SHOWN ON ARCHITECTURAL DRAWINGS: SIZES AND LOCATIONS OF WINDOWS AND DOOR OPENINGS, CONCRETE CURBS, FLOOR DRAINS AND DEPRESSSED SLAB AREAS, FLOOR AND INTERIOR OR EXTERIOR NON-STRUCTURAL WALLS/PARTITIONS, ETC.
- J. THE CONTRACTOR SHALL BE DIRECTLY RESPONSIBLE FOR COORDINATING THE FOLLOWING TYPES OF ITEMS WHICH ARE TYPICALLY SHOWN ON MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS: SIZES AND LOCATIONS OF MECHANICAL EQUIPMENT, DUCT/WORK RUNS, CONDUIT OR CABLE TRAY RUNS, PIPE RUNS, AND ALL ASSOCIATED SLEEVES, PENETRATIONS, OPENINGS, HANGERS, INSERTS, ETC.
- K. THE CONTRACTOR SHALL BE DIRECTLY RESPONSIBLE FOR COORDINATING THE STRUCTURE (I.E. LENDING, FOOTINGS, ETC.) TO WORK WITH GRADES WHICH ARE TYPICALLY SHOWN ON ARCHITECTURAL OR CIVIL DRAWINGS. THE STRUCTURAL ENGINEER TAKES NO RESPONSIBILITY FOR DAMAGE TO THE STRUCTURE, OR ADDITIONAL CONSTRUCTION COSTS CAUSED BY, LACK OF SUCH COORDINATION OR MISREPRESENTATIONS OF THE RELATIVE POSITION OF THE STRUCTURE WITH RESPECT TO GRADES ON THE BUILDING SITE.
- EARTHWORK
- A. EARTHWORK REQUIREMENTS AND FOUNDATIONS DESIGNS ARE BASED ON COMMON LOCAL PRESUMPTIVE SOIL DESIGN VALUES AND ASSUME THE PRESENCE OF ADEQUATE NATIVE SOILS.
- B. WHERE REQUIRED, A SOILS ENGINEER SHALL BE RETAINED TO REVIEW AND APPROVE ALL EARTHWORK MATERIALS PRIOR TO PLACEMENT. OBSERVE EARTHWORK OPERATIONS TO VERIFY COMPLIANCE WITH EARTHWORK REQUIREMENTS, AND PROVIDE DIRECTION WHERE UNEXPECTED CONDITIONS ARISE.
- C. STRIP SITE, CLEAR ALL DEBRIS, PAVING AND ORGANICS, AND EXCAVATE AS REQUIRED TO CONSTRUCT IMPROVEMENTS, ALLOWING ADEQUATE SPACE WHERE NECESSARY TO CONSTRUCT AND REMOVE FORMS.
- D. AFTER EXCAVATIONS ARE COMPLETE AND PRIOR TO PLACEMENT OF ANY FILL OR FOUNDATIONS, SCARIFY, MOISTURE CONDITION AND RE-COMPACT SUBGRADES TO 90% RELATIVE COMPACTION, UNLESS OTHERWISE DIRECTED BY THE SOILS ENGINEER, WHERE SOIL STABILIZATION MEASURES ARE REQUIRED, THEY SHALL BE AS DIRECTED BY THE SOILS ENGINEER.
- E. EXISTING SOILS MAY BE ACCEPTABLE AS ENGINEERED FILL PROVIDED THE MATERIAL IS SCREENED AND BLENDED TO MEET THE CRITERIA PROVIDED BY A SOILS ENGINEER. COORDINATE ALL SOIL FILL AND BACKFILL REQUIREMENTS WITH THE SOILS ENGINEER.
- F. PLACE FILL AND BACKFILL IN UNIFORM HORIZONTAL LIFTS OF 8" MAXIMUM LOOSE THICKNESS AND COMPACT TO 90% RELATIVE COMPACTION UNLESS OTHERWISE DIRECTED BY THE SOILS ENGINEER.
- G. ALL CONCRETE SLABS-ON-GRADE, STEPS AND FLATWORK SHALL BE UNDERLAIN WITH AGGREGATE BASE CONFORMING TO NEVADA HIGHWAY DEPARTMENT SPECIFICATIONS FOR TYPE 2, CLASS B AGGREGATE BASE. COMPACT AGGREGATE BASE TO 95% RELATIVE COMPACTION.
- H. TRENCHING AND BACKFILL FOR UTILITIES SHALL COMPLY WITH THE REQUIREMENTS OF THE AUTHORITIES HAVING JURISDICTION AND WITH ALL BEST INDUSTRY PRACTICES.
- I. BOTTOMS OF ALL FOUNDATIONS SHALL BE LEVEL. CHANGES IN BOTTOM OF FOUNDATION ELEVATION SHALL BE MADE ACCORDING TO TYPICAL STEPPED FOOTING DETAILS PROVIDED IN THE DRAWINGS.
- J. COORDINATE REQUIREMENTS FOR WATERPROOFING OF FOUNDATIONS, STEMWALLS OR RETAINING WALLS WITH THE ARCHITECT AND/OR OWNER.
- K. CONTRACTOR SHALL PROVIDE DESIGN AND INSTALLATION OF ANY AND ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY AND ADEQUATELY RETAIN EARTH BANKS WHERE REQUIRED.
- L. CONTRACTOR SHALL COMPLY WITH LOCAL ORDINANCES FOR DUST CONTROL AND SHALL MAINTAIN TEMPORARY DRAINAGE ROUTES TO KEEP SURFACE WATER OUT OF EXCAVATIONS. PUMP FOOTING EXCAVATIONS IF WATER ACCUMULATES. CONTRACTOR SHALL PROVIDE FOR PROPER DE-WATERING OF EXCAVATIONS FROM SURFACE WATER, GROUND WATER, SEEPAGE, ETC.
- M. ALL FOOTINGS SHALL BE FOUNDED A MINIMUM OF 24" BELOW ADJACENT EXTERIOR FINISHED GRADE TO PROVIDE BURY FOR FROST DEPTH. FOOTINGS MAY BE POURED IN NEAT EXCAVATION WHERE POSSIBLE, PROVIDED AN EXTRA 1" WIDTH OF FOOTING IS PROVIDED ON EACH SIDE. FOOTING EXCAVATIONS SHALL BE HAND CLEANED PRIOR TO PLACEMENT OF CONCRETE.

CAST-IN-PLACE CONCRETE

- A. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ACI MANUAL OF CONCRETE PRACTICE. CONCRETE MIX DESIGNS SHALL BE IN ACCORDANCE WITH ASTM C-94 AS FOLLOWS:
- FOUNDATIONS & INTERIOR SLABS ON GRADE: Fc = 4000 PSI AT 28-DAYS, 3/4" NORMAL WEIGHT AGGREGATE, 0.45 MAXIMUM W/C RATIO, ENTRAPPED AIR ONLY. SLABS ON GRADE SHALL CONTAIN 1.5 LB/CY OF FIBERMESH STEALTH POLYPROPYLENE FIBERS, SOLOMON ULTRAFIBER 300 CELLULOSE FIBERS, OR APPROVED EQUAL.
 - EXTERIOR CONCRETE: Fc = 4800 PSI AT 28-DAYS, 3/4" NORMAL WEIGHT AGGREGATE, 0.45 MAXIMUM W/C RATIO, 6% +/- 1% ENTRAPPED AIR. EXTERIOR CONCRETE SHALL CONTAIN 1.5 LB/CY OF FIBERMESH STEALTH POLYPROPYLENE FIBERS OR APPROVED EQUAL.
- B. CONCRETE DESIGN ON THIS PROJECT IS BASED ON Fc = 2500 PSI SO SPECIAL INSPECTION IS NOT REQUIRED.
- C. ALL CONCRETE MIXES SHALL UTILIZE ASTM C150 TYPE II LOW ALKALI CEMENT. ASTM C618 CLASS F FLY ASH OR APPROVED NATURAL POZZOLAN MAY BE UTILIZED FOR UP TO 25% CEMENT REPLACEMENT AT THE CONTRACTOR'S OPTION.
- D. NORMAL WEIGHT AGGREGATE SHALL CONFORM TO ASTM C33 AND LIGHTWEIGHT AGGREGATE SHALL CONFORM TO ASTM C330. ALL AGGREGATE SHALL BE FROM APPROVED SOURCES AND FREE OF DELETERIOUS MATERIALS.
- E. CONCRETE SLUMP SHALL NOT EXCEED 3" WHEN TESTED IN ACCORDANCE WITH ASTM C143 FOR HORIZONTAL MEMBERS SUCH AS FOOTINGS, SLABS, AND BEAMS OR 4" FOR VERTICAL MEMBERS SUCH AS WALLS AND COLUMNS. CONCRETE SHALL BE PLACED AT THE MINIMUM PRACTICAL SLUMP, NOT EXCEEDING THE SPECIFIED MAXIMUM SLUMP. IF ADDITIONAL WORKABILITY AND SLUMP IS DESIRED, IT MAY BE OBTAINED WITH APPROVED ADMIXTURES THAT DO NOT INCREASE WATER CONTENT, SHRINKAGE, OR ADVERSELY AFFECT THE CONCRETE.
- F. ALL NON-SHRINK GROUT AND DRYPACK SHALL BE A PREMIXED, NON-METALLIC, SHRINKAGE COMPENSATING, NON-STAINING FORMULA WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 3 DAYS AND 7000 PSI AT 28 DAYS. GROUT SHALL BE MIXED AND PLACED PER ALL MANUFACTURER REQUIREMENTS IN A FLOWABLE OR PACKABLE STATE AS REQUIRED BY THE CONDITIONS OF INSTALLATION. USE MASTER-BUILDERS' CONSTRUCTION GROUT OR APPROVED EQUAL.
- G. REINFORCING SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60. ALL BARS TO BE WELDED OR FIELD BENT SHALL BE ASTM A108. ALL WELDING SHALL CONFORM TO AWS D1.4 AND ALL WELDING SHALL BE PERFORMED BY APPROPRIATELY CERTIFIED WELDERS. SUPPORTS AND ACCESSORIES FOR REINFORCING SHALL BE FURNISHED AS SHOWN OR REQUIRED. CHAIRS PLACED AGAINST EXPOSED SURFACES SHALL BE GALVANIZED, STAINLESS, OR PLASTIC.
- H. WELDED WIRE FABRIC (W/WF) WITH PLAN REINFORCING WIRE SHALL CONFORM TO ASTM A108. STRUCTURAL WELDED WIRE REINFORCING (S/WWR) WITH DEFORMED REINFORCING SHALL CONFORM TO ASTM A477. W/WF AND S/WWR SHALL HAVE WIRE SIZE AND SPACING AS INDICATED ON PLANS AND DETAILS. LAP SHEETS IN ACCORDANCE WITH ACI AND CRSI RECOMMENDATIONS, 1-1/2 MESHES MINIMUM.
- I. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN CONFORMANCE WITH ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND THE CRSI MANUAL OF STANDARD PRACTICE. ALL REINFORCING BAR BENDS SHALL BE MADE COLD. ALL REINFORCING, DOVELS, BOLTS, ANCHORS, SLEEVES, ETC. SHALL BE ACCURATELY POSITIONED AND SECURED IN PLACE WITH CHAIRS, TIES, BOLSTERS OR DOBIES PRIOR TO PLACEMENT OF CONCRETE, NO "WET-SETTING" IS ALLOWED.
- J. COVERAGE FOR REINFORCING SHALL BE THE CLEAR DISTANCE FROM FACE OF CONCRETE TO THE FACE OF NEAREST BARS AS FOLLOWS, UNLESS NOTED OTHERWISE:
- CAST AGAINST PERMANENT CONTACT WITH GROUND (EXCEPT SLABS): 3"
 - EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
 - 2" FOR 16 AND LARGER BARS
 - 1-1/2" FOR 14 AND SMALLER BARS
 - NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
 - SLABS, JOISTS AND WALLS: 1-1/2" FOR #14 AND #18 BARS
 - SLABS, JOISTS AND WALLS: 3/4" FOR #11 AND SMALLER BARS
 - BEAMS, COLUMNS, PEDESTALS AND TENSION TIES: 1-1/2"
 - SLABS-ON-GRADE: 1-1/2" CLR FROM TOP
- K. REINFORCING SPLICES SHALL BE MADE ONLY WHERE INDICATED ON THE DRAWINGS UNLESS PRIOR APPROVAL IS OBTAINED FROM THE A/E. REINFORCING BARS SHALL BE SPLICED A MINIMUM OF 48 DIAMETERS OR AS NOTED, IN NO CASE SHALL SPLICES BE LESS THAN 24".
- L. CONSOLIDATE CONCRETE PLACED IN FORMS BY MECHANICAL VIBRATING EQUIPMENT, SUPPLEMENTED BY HAND-SPADING, RODDING OR TAMPING. USE EQUIPMENT AND PROCEDURES FOR CONSOLIDATION OF CONCRETE IN ACCORDANCE WITH THE RECOMMENDED PRACTICES OF ACI 309 TO SUIT THE TYPE CONCRETE AND PROJECT CONDITIONS. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL, AS IN WALLS, SO AS TO CAUSE SEGREGATION. IN SUCH CASES HOPPERS AND CHUTES OR TRUNKS OF VARIABLE LENGTHS SHALL BE USED TO LIMIT FREE UNCONFINED FALL OF CONCRETE TO 6 FEET.
- M. ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- N. DESIGN CONCRETE FORMS AS RECOMMENDED IN ACI 347. CONSTRUCT FORMS OF ADEQUATE STRENGTH AND STIFFNESS TO OBTAIN REQUIRED FINISHES AND CONCRETE SURFACE AND LINE TOLERANCES SPECIFIED IN ACI 111. FORMS SHALL BE TIGHT ENOUGH TO PREVENT LEAKAGE OF MORTAR FINES.
- O. USE NEW OR PROPERLY CLEANED LIKEN-FORM MATERIALS. REMOVE ALL DIRT, CHIPS, SANDUST, RUBBISH, WATER OR ICE FROM FORMS PRIOR TO PLACEMENT OF CONCRETE.
- P. CLEAN AND ROUGHEN CONSTRUCTION JOINTS AND LIGHTLY MOISTEN FORMS AND SUBGRADE PRIOR TO PLACING CONCRETE. INSTALL WIRE MESH "DEBRIS DUGOUT" CHEMICAL RELEASE AGENT OR APPROVED EQUAL PRIOR TO PLACEMENT OF CONCRETE. PLACE CONCRETE USING METHODS WHICH AVOID SEGREGATION. MECHANICALLY VIBRATE ALL CONCRETE, INCLUDING SLABS, TO CONSOLIDATE IT IN FORMS. COMPLY WITH THE REQUIREMENTS FOR CURING PRIOR TO STRIPPING OF FORMS.
- Q. CONCRETE FINISHES:
- INTERIOR FLATWORK: SCREED TO AN EVEN, LEVEL PLANE, FLOAT AND STEEL TROWEL TO A SMOOTH, DENSE, HARD FINISH.
 - EXTERIOR FLATWORK: AS ABOVE EXCEPT FOLLOWED WITH A MEDIUM BROOM FINISH PERPENDICULAR TO TRAFFIC.
 - CORNER SURFACES EXPOSED TO VIEW IMMEDIATELY AFTER FORMS ARE REMOVED, REMOVE SURFACE PROJECTIONS AND SACK AND PATCH ALL SURFACE DEFECTS.
 - FORMED SURFACES NOT EXPOSED TO VIEW: STRIP FORMS AND PATCH LARGE HOLES OR DEFECTS.
 - COORDINATE AND VERIFY ALL CONCRETE FINISHES WITH THE ARCHITECT.
- R. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" OR TOOLED TO A 1/2" RADIUS.
- S. FINISH AND MEASURE SLAB SURFACES 50 G/P AT ANY POINT BETWEEN CONCRETE SURFACE AND AN UNLEVELLED, FREESTANDING, 10-FOOT LONG STRAIGHTEDGE RESTING ON TWO HIGH SPOTS AND PLACED ANYWHERE ON THE SURFACE DOES NOT EXCEED 1/4".
- T. CONCRETE CURING AND PROTECTION REQUIREMENTS:
- FRESHLY DEPOSITED CONCRETE SHALL BE CURED AND PROTECTED FROM PREMATURE DRYING AND EXCESSIVE HOT OR COLD TEMPERATURES FOR A MINIMUM OF 5 DAYS IN ACCORDANCE WITH ACI 308 IN HOT WEATHER AND ACI 306 IN COLD WEATHER.
 - INTERIOR SLABS TO BE COVERED: APPLY 1 COAT OF ASTM C309 CURING COMPOUND AS SOON AS POSSIBLE AFTER FINISHING. CURING COMPOUND SHALL BE CERTIFIED TO BE COMPATIBLE WITH FLOORING COVERINGS OR THE CURING COMPOUND SHALL BE REMOVED FROM SLABS PRIOR TO APPLICATION OF FLOOR COVERINGS.
 - INTERIOR SLABS TO BE EXPOSED WITH CURE AND SEAL FINISH: APPLY AN INITIAL CURING COAT OF HIGH SOLIDS (20% MINIMUM) ASTM C1315 CURING AND SEALING COMPOUND AS SOON AS POSSIBLE AFTER FINISHING. APPLY A SECOND SEALING COAT PER MANUFACTURER RECOMMENDATIONS.
 - EXTERIOR CONCRETE: APPLY 1 COAT OF HIGH SOLIDS (20% MINIMUM) ASTM C309 CURING COMPOUND AS SOON AS POSSIBLE AFTER FINISHING.
 - REAPPLY CURING COMPOUNDS TO SAWCUT JOINTS IMMEDIATELY AFTER CUTTING IF CURING COMPOUND IS COMPLETELY PRIOR TO CUTTING. ALL SAW CUTTING SHALL BE TIED TO AVOID TEARING OR DAMAGE BY THE SAW BLADE.
- U. DEFECTIVE WORK:
- ANY CONCRETE NOT FORMED AS SHOWN OR NOT MEETING THE INTENDED LINES, ELEVATIONS, FINISHES, TOLERANCES, ETC. SHALL BE DEEMED DEFECTIVE.
 - SLAB CRACKS, EDGE CURLING AND SURFACES NOT MEETING FINISH, FLATNESS OR LEVELNESS REQUIREMENTS SHALL BE DEEMED DEFECTIVE.
 - DEFECTIVE WORK SHALL BE REMOVED AND REPLACED WITH CONFORMING WORK, OR AT THE OPTION OF THE A/E, REPAIRED TO THE SATISFACTION OF THE A/E.
- V. CONSTRUCTION JOINTS WILL NOT BE PERMITTED, EXCEPT WHERE SHOWN ON THE DRAWINGS, WITHOUT WRITTEN CONSENT OF THE A/E.
- W. ALL CONDUITS AND UTILITIES AT SLABS ON GRADE SHALL BE PLACED IN THE BASE MATERIALS AND NOT IN THE SLAB.
- X. LEAVE CONCRETE SURFACES BROOM CLEAN AND REMOVE ALL DEBRIS FROM CONCRETE WORK FROM THE SITE.

ROUGH CARPENTRY

- A. FRAMING LUMBER SHALL BE DOUGLAS FIR WITH A MAXIMUM MOISTURE CONTENT OF 19% AT THE TIME OF DELIVERY TO THE SITE. ALL FRAMING EXPOSED TO VIEW SHALL BE KILN DRIED. ALL LUMBER SHALL BE FREE OF HEART CROTCHES TYPICALLY. FRAMING LUMBER SHALL BE AS SPECIFIED BELOW AS A MINIMUM UNLESS NOTED OTHERWISE.
- PLATES, BRIDGING AND BLOCKING: DF-L #2
 - 2x AND 3x STUDS: DF-L #2
 - 4x AND LARGER STUDS AND POSTS: DF-L #1
 - 2x AND 3x FRAMING, UP TO 8" NOMINAL DEPTH: DF-L #2
 - 2x AND 3x FRAMING, OVER 8" NOMINAL DEPTH: DF-L #1
 - 4x AND LARGER FRAMING: DF-L #1
- B. MANUFACTURED LUMBER PRODUCTS SHALL BE AS MANUFACTURED BY "TRUE JOIST" OR APPROVED EQUAL AND SHALL BE AS SPECIFIED BELOW UNLESS NOTED OTHERWISE. DESIGN AND FABRICATION SHALL CONFORM TO THE CURRENT BUILDING CODE AND ICC REQUIRED REQUIREMENTS.
- PARALLEL STRAND LUMBER (PSL): 2.0E, Fb = 2400 PSI, Fv = 240 PSI
 - LAMINATED VENEER LUMBER (LVL): 2.0E, Fb = 2600 PSI, Fv = 285 PSI
 - LAMINATED STRAND LUMBER (LSL): 1.55E, Fb = 2325 PSI, Fv = 400 PSI
- C. PRE-MANUFACTURED WOOD CHORD JOISTS WITH PLYWOOD OR ROD KEEPS (I.E. T-JL OR T-JI) SHALL BE THE TYPES AND SIZES INDICATED ON THE DRAWINGS.
- D. CUT FRAMING MEMBERS SQUARE AND TO ACCURATE LENGTH TO OBTAIN FULL BEARING AT JOINTS. ERECT PLUMB AND LEVEL AND TO ACCURATE LINE. BOLT HOLES SHALL BE ACCURATELY DRILLED 1/16" LARGER THAN BOLT SIZES. BOLTS SHALL CONFORM TO ASTM A307 WITH STANDARD CUT WASHERS. WHERE HEAD OR NUT BEARS AGAINST WOOD, SELECT MEMBERS 50 KNOTS OR DEFECTS DO NOT OCCUR AT NAIL OR BOLT LOCATIONS.
- E. DO NOT NOTCH FRAMING, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN THE ENGINEER'S APPROVAL FOR ANY HOLES OR NOTCHES NOT DETAILED. HOLES THROUGH SILLS, PLATES, STUDS, AND DOUBLE PLATES OF BEARING OR SHEAR WALLS SHALL BE BORED IN THE CENTER OF THE MEMBER AND SHALL NOT EXCEED 1/3 THE MEMBER WIDTH.
- F. NAILING SHALL BE IN ACCORDANCE WITH THE FASTENING SCHEDULE IN BUILDING CODE CHAPTER 23 WHERE NOT OTHERWISE SHOWN ON DRAWINGS.
- G. FRAMING ANCHORS, STRAPS, CONNECTIONS, HANGERS, ETC., SHALL BE SIMPSON STRONG TIE, OR APPROVED EQUAL HAVING ICC APPROVAL. PREDRILL NAIL HOLES AS REQUIRED TO AVOID SPLITTING. ALL HANGERS AND CONNECTIONS SHALL BE FASTENED FOR MAXIMUM CAPACITY.
- H. PLATES, LEDGERS, ETC. ATTACHED DIRECTLY TO CONCRETE OR MASONRY AT GRADE SHALL BE PRESSURE TREATED AND CONNECTORS FOR PRESSURE TREATED LUMBER SHALL BE GALVANIZED OR STAINLESS AS APPROPRIATE FOR THE PRESSURE TREATED MATERIAL BEING FASTENED. FIELD CUTS AND BOLT HOLES IN PRESSURE TREATED LUMBER SHALL BE PROTECTED IN ACCORDANCE WITH ANPA STANDARD M4.
- I. ALL CONNECTORS AND FASTENERS IN CONTACT WITH PRESSURE TREATED OR FIRE-RETARDANT-TREATED LUMBER THAT IS NOT "TIMBERSTRAND LSL TREATED" OR "BORATE TREATED" SHALL EITHER BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER ZINC COATING AND SHALL CONFORM TO ASTM A-153.
- J. ALL SOLE PLATE ANCHOR BOLTS TO HAVE MINIMUM 3"x3"x1/4" PLATE WASHERS AND THE WASHER EDGE SHALL BE WITHIN 1/2" OF SHEAR PLYWOOD WHERE OCCURS. IN LOCATIONS OF DOUBLE SIDED SHEATHING, A RECTANGULAR SIMPSON BR5-6 OR EQUIVALENT WASHER SHALL BE USED TO ADDRESS THE 1/2" MAXIMUM REQUIRED DISTANCE FROM SHEAR PLYWOOD. IF SLOTTED PLATE WASHERS ARE USED, A STANDARD CUT WASHER SHALL ALSO BE USED ON TOP OF THE SLOTTED PLATE WASHER.
- K. SHEATHING SHALL CONFORM STRUCTURALLY TO APA STANDARDS AND TO U.S. PRODUCT STANDARD PS-1. ALL PIECES SHALL BE GRADE STAMPED AND SHALL BE OF THE GRADES AND SIZES SHOWN ON THE DRAWINGS. OSB SHALL NOT BE USED IN CONDITIONS WITH EXTERIOR EXPOSURE.
- METAL-PLATE-CONNECTED WOOD TRUSSES
- A. FACTORY FABRICATED METAL PLATE CONNECTED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH ANSI / TPI 1. SHOP DRAWINGS STAMPED BY A LICENSED PROFESSIONAL ENGINEER, REGISTERED WITH THE STATE THE PROJECT OCCURS, SHALL BE SUBMITTED TO THE A/E FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION. SHOP DRAWINGS SHALL ALSO INCLUDE MANUFACTURER'S INSTALLATION INSTRUCTIONS ON LATERAL BRACING AND ALL REQUIRED HANGERS.
- B. DESIGN TRUSSES FOR THE FOLLOWING LOADS:
- DEAD LOAD = 34 PSF (GROW)
 - TG DEAD LOAD = 10 PSF
 - BG LIVE LOAD = 10 PSF (NON-CONCURRENT WITH SNOW)
 - BG DEAD LOAD = 10 PSF
 - SEE ROOF FRAMING PLANS FOR SPECIAL LOADS. TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND FORCES. SEE DESIGN CRITERIA FOR ADDITIONAL INFORMATION, NO MORE THAN 10 PSF TOTAL DEAD LOAD SHALL BE USED TO RESIST WIND UPLIFT LOADS. NO STRESS INCREASES SHALL BE ALLOWED FOR REPETITIVE MEMBERS.
- C. LIMIT TRUSS DEFLECTIONS TO L/360 FOR LIVE LOADS AND L/240 FOR TOTAL LOADS.
- D. SEE DRAWINGS FOR TRUSS CONFIGURATIONS. DIAGONAL WEB MEMBERS MAY BE LOCATED PER THE TRUSS FABRICATOR'S REQUIREMENTS, HOWEVER, WHERE DIFFERENT TRUSS TYPES ARE LOCATED IN A SINGLE RUN, WEB MEMBERS AND PANEL POINTS SHALL LINE UP SO THAT A CLEAR PASSAGE IS MADE AVAILABLE FOR MECHANICAL DUCT WORK, ETC.
- E. MINIMUM MEMBER SIZES SHALL BE 2x6 TOP CHORDS AND 2x4 FOR ALL OTHER MEMBERS. USE DOUG-FIR LARCH FOR TOP CHORD MEMBERS. USE DOUG-FIR LARCH OR HEM-FIR LUMBER OR BETTER FOR OTHER MEMBERS. USE "S-DRY" OR "KILN-DRY" LUMBER WITH 19% OR LESS MOISTURE CONTENT WHEN FABRICATED.
- F. TRUSSES SHALL BE MARKED BY THE FABRICATOR AT LOCATIONS REQUIRING CONTINUOUS BRACING. CROSS BRACING SHALL BE FURNISHED AND INSTALLED AS INDICATED BY THE TRUSS MANUFACTURER AND AS SHOWN ON THESE DRAWINGS.
- G. CONNECTORS BETWEEN TRUSS ELEMENTS ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER, PER BUILDING CODE SECTION 2303.4.1.1
- H. BUILT-UP GIRDER TRUSSES, GREATER THAN 3 LAMINATIONS, SHALL BE LAMINATED USING 1/2" DIAMETER BOLTS AT 24" O.C. MAX THROUGH ALL MEMBERS.
- I. GABLE END TRUSSES SHALL HAVE 2x VERTICALS AT 16" O.C., TYPICAL UNLESS NOTED OTHERWISE.
- J. TRUSS DESIGNER TO ACCOUNT FOR THE WEIGHT OF ALL MECHANICAL EQUIPMENT IN THE DESIGN OF ALL TRUSSES WHICH SUPPORT SUCH UNITS. SEE MECHANICAL DRAWINGS FOR WEIGHTS AND DETAILING.
- K. TRUSS MANUFACTURER TO ALLOW FOR ATTIC ACCESS, WHERE APPLICABLE, PER PLAN. IF ACCESS IS NOT SHOWN ON PLANS, TRUSS MANUFACTURER SHALL VERIFY AND COORDINATE LOCATION WITH A/E.
- L. CAMBER TRUSSES FOR 1.5 TIMES THE DEAD LOAD DEFLECTION UNLESS NOTED OTHERWISE.

MASONRY

- A. ALL MASONRY WORK SHALL CONFORM TO THE REQUIREMENTS OF TMS 402 BUILDING CODE REQUIREMENTS AND TMS 602 SPECIFICATION FOR MASONRY STRUCTURES. MINIMUM DESIGN STRENGTH Fm = 2000 PSI.
- B. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C40 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS AND SHALL BE MADE WITH LIGHTWEIGHT AGGREGATE. MASONRY UNITS SHALL BE OF THE SIZE, SHAPE, COLOR, TEXTURE AND LAYOUT AS DIRECTED AND APPROVED BY THE ARCHITECT. USE OPEN END UNITS WHEREVER POSSIBLE. USE BOND BEAM UNITS AT HORIZONTAL REINFORCING.
- C. MORTAR SHALL CONFORM TO ASTM C270, TYPE M WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS. MORTAR COLOR SHALL BE AS DIRECTED AND APPROVED BY THE ARCHITECT.
- D. GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. GROUT SHALL BE COARSE GROUT IN WALLS 8" AND LARGER AND FINE GROUT IN 6" WALLS OR WHERE CONDITIONS WARRANT.
- E. REINFORCING SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. ALL REINFORCING TO BE WELDED SHALL CONFORM TO ASTM A108. ALL REINFORCING BARS SHALL BE ACCURATELY DETAILED, POSITIONED AND HELD SECURELY IN PLACE PRIOR TO GROUTING, AT TOP AND BOTTOM AND INTERVALS BETWEEN NOT-TO-EXCEED 142 DIAMETERS.
- F. BARS SHALL BE LAPPED A MINIMUM OF 48 DIAMETERS. LAP SHALL BE A MINIMUM OF 12 BAR DIAMETERS WHERE BARS ARE LOCATED 3" OR LESS FROM FACES OF MASONRY AND WHERE MULTIPLE BARS ARE LAPPED WITHIN A SINGLE CELL. BARS SHALL MAINTAIN 1/2" MINIMUM CLEARANCE TO FACE SHELLS FOR GROUTING.
- G. REINFORCING SHALL TAKE PRECEDENCE OVER THE LOCATION OF CONDUITS IN ALL CASES AND ADEQUATE ROOM SHALL BE MAINTAINED IN ALL CASES TO ALLOW GROUT TO FLOW COMPLETELY AROUND REINFORCING AND CONDUIT. THE FOLLOWING REQUIREMENTS SHALL APPLY TO CONDUITS PLACED IN MASONRY UNLESS DIRECTED OTHERWISE BY THE A/E.
- PLACE CONDUIT IN NON-REINFORCED CELLS OR BOND BEAMS WHEREVER POSSIBLE.
 - NO CONDUIT SHALL BE PLACED WITHIN 8" OF ANY JAMB, WALL END, HEAD, SILL OR TOPS OF WALLS. CELLS AND BOND BEAMS WITHIN 8" OF JAMBS, WALL ENDS, HEADS, SILLS AND TOPS OF WALLS SHALL BE RESERVED FOR REINFORCEMENT ONLY.
 - CELLS OR BOND BEAMS CONTAINING MORE THAN (1) REINFORCING BAR: 8" CMU - NO CONDUIT SHALL BE PLACED; 12" CMU - NO MORE THAN (1) 1-3/4"Ø MAX. (OD) CONDUIT.
 - CELLS OR BOND BEAMS REINFORCED WITH A SINGLE BAR: 8" CMU - NO MORE THAN (1) 1"Ø MAX. (OD) CONDUIT EXCEPT (1) 1-3/4"Ø MAX. (OD) CONDUIT WILL BE ALLOWED IN THE BOTTOM 4" A.F.F.; 12" CMU - NO MORE THAN (2) 1"Ø MAX. (OD) OR (1) 1-3/4"Ø MAX. (OD) CONDUIT.
 - NON-REINFORCED CELLS OR BOND BEAMS: 8" CMU - NO MORE THAN (2) 1" MAX. (OD) CONDUITS; 12" CMU - NO MORE THAN (2) 1-3/4"Ø MAX. (OD) CONDUIT.
 - LAYOUT CONDUIT AND PROVIDE ALL NECESSARY PROVISIONS, SUCH AS ADDITIONAL ROWS OF BOND BEAM UNITS, TO MAINTAIN THE REQUIREMENTS NOTED ABOVE. MAINTAIN 1/2" MIN. GROUT AROUND ALL CONDUIT AND REBAR.

- H. BEFORE MASONRY IS PLACED ON CONCRETE, THOROUGHLY CLEAN CONCRETE OF ALL LANTANE AND LOOSE MATERIAL AND ROUGHEN SURFACE.
- I. LAY BLOCK IN RUNNING BOND IN 3/8" FULL SHOVED HEAD AND BED JOINTS AND TOOL ALL JOINTS AS CONVEX WEATHER RESISTANT JOINTS UNLESS DIRECTED OTHERWISE BY THE ARCHITECT. CONTRACTOR TO COORDINATE WITH ARCHITECTURAL DRAWINGS FOR ALL MASONRY TYPES, DIMENSIONS AND LAYOUT PATTERNS PRIOR TO CONSTRUCTION. LAY UNITS TO PRESERVE THE UNOBSTRUCTED VERTICAL CONTINUITY OF CELLS.
- J. ALL CELLS SHALL BE GROUTED SOLID IN 8'-0" MAXIMUM LIFTS, SPLICES OF REINFORCEMENT SHALL BE LOCATED SUCH THAT THE FULL SPLICE LENGTH IS WITHIN A LIFT AND IS OBSERVABLE AND VERIFIABLE PRIOR TO GROUTING. GROUT, CUT CLEANOUTS SHALL BE PROVIDED AT THE BOTTOM OF EACH LIFT FOR CLEANING AND INSPECTION. AT THE CONTRACTOR'S OPTION, LIFTS MAY BE REDUCED TO 5'-0" MAXIMUM WITH CLEANOUTS OMITTED. ALL GROUT SHALL BE VIBRATED AND REVIBRATED USING INTERNAL MECHANICAL VIBRATORS. KEY ALL GROUT POURS BY HOLDING TOP OF GROUT APPROXIMATELY 2" BELOW TOP OF BLOCK.
- K. PROTECT MASONRY WORK AS REQUIRED BY ACI 308.1 FOR COLD WEATHER, HOT WEATHER AND MOISTURE.

STANDARD ABBREVIATIONS

AB	ANCHOR BOLT	JG	STEEL JOIST GIRDER
A/E	ARCHITECT / ENGINEER	JT	Joint
AFE	ADJUVANT FOR	K	KING STUD, KIPS
AGG	AGGREGATE	KSI	KIPS PER SQUARE INCH
ALT	ALTERNATE	KSF	KIPS PER SQUARE FOOT
ALUM	ALUMINUM	LB	POUNDS
APPROX	APPROXIMATELY	LLV	LONG LEG VERTICAL
ARCH	ARCHITECT	LLH	LONG LEG HORIZONTAL
BFL	BELOW FIN FLR	LT WT	LIGHT WEIGHT
BLDG	BUILDING	MAX	MAXIMUM
BLKS	BLOCKING	MB	MACHINE BOLT
BM	BEAM	MECH	MECHANICAL
BN	BOUNDARY NAIL	MFR	MANUFACTURER
BO	BOTTOM OF	MISC	MISCELLANEOUS
BOT	BOTTOM	MIN	MINIMUM
B/S	BOTH SIDES	MTL	METAL
CF	CAST IN PLACE	N	NOT
CJP	COMPLETE JOINT	NA	NOT APPLICABLE
CL	CEILING	NS	NEAR SIDE
CLR	CLEAR	NSA	NEAR END STUD ANCHOR
CLM	CLIMB	NTS	NOT TO SCALE
CLN	COLUMN	OC	ON CENTER
CLC	CLEAR	OD	OUTSIDE DIAMETER
CMU	CONC MASONRY UNIT	OH	OPPOSITE HAND
CONC	CONCRETE	OJW	OPEN WEB STEEL JOIST
CONN	CONNECTION	PERP	PERPENDICULAR
CONST	CONSTRUCTION	PLF	POUNDS PER FOOT
CONT	CONTINUOUS	PLY	PLYWOOD
CTSK	COUNTERSUNK	PSI	POUNDS PER SQUARE INCH
CUR	CURB	PSF	POUNDS PER SQUARE FOOT
DBL	DOUBLE	PT	PRESSURE TREATED
DFL	DOUGLAS FIR	RDG	REQUIRED
DIA	DIAMETER	RENF	REINFORCING
DJL	DOWELED JOINT	REQD	REQUIRED
DNG	DRAWING	SCH	SCHEDULE
EA	EACH	SHTS	SHEATHING
EF (E/F)	EACH FACE	SMR	SMILAR
ELEV	ELEVATION	SOB	SLAB ON GRADE
ELEC	ELECTRICAL	SS	STAINLESS STEEL
EDG	EDGE NAIL	STAGG	STAGGERED
ECR	ENGINEER OF RECORD	STD	STANDARD
EA (E/W)	EACH WAY	STIFF	STIFFENER
EX	EXISTING	STL	STEEL
EXT	EXTERIOR	STRUC	STRUCTURAL
FF	FINISHED FLOOR	SYM	SYMMETRICAL
FIN	FINISH	T	TRIMMER
FLR	FLOOR	TN	TOENAIL
FN	FACE NAIL	TOP	TOP OF
FO	FACE OF	TYP	TYPICAL
FS	FAR SIDE	T&G	TONGUE & GROOVE
FT	FEET	T&B	TOP & BOTTOM
FTG	FOOTING	UN	UNLESS NOTED OTHERWISE
GA	GAGE, GAUGE	VERT	VERTICAL
GALV	GALVANIZED	VIF	VERIFY IN FIELD
GUB	GULF	VNF	FIELD FLANGE
HDR	HEADER	W/F	WELDED WIRE FABRIC
HORIZ	HORIZONTAL	W/O	WITH/OUT
HSD	HIGH STRENGTH BOLT	W	WITH
ID	INSIDE DIAMETER	#	AT
IN	INCHES	#	NUMBER, POUNDS
INT	INTERIOR	+/-	PLUS OR MINUS

SHEET INDEX		
Sheet Number	Sheet Name	Sheet Issue Date
S0.1	STRUCTURAL COVER SHEET & NOTES	11/10/2022
S0.2	TYPICAL DETAILS	11/10/2022
S0.3	TYPICAL DETAILS	11/10/2022
S1.1	FOUNDATION AND SHEAR PLANS	11/10/2022
S2.1	MAIN ROOF FRAMING PLAN	11/10/2022
S3.1	FOUNDATION AND ROOF FRAMING DETAILS	11/10/2022

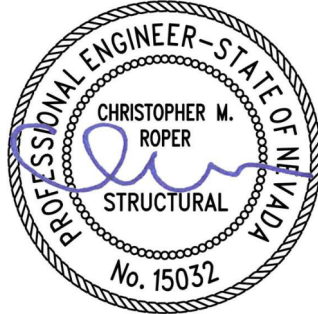
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5425 LOUIE LANE, RENO, NV

project

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Johnson Lane Park Storage Building
Douglas County Community Development
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revisions

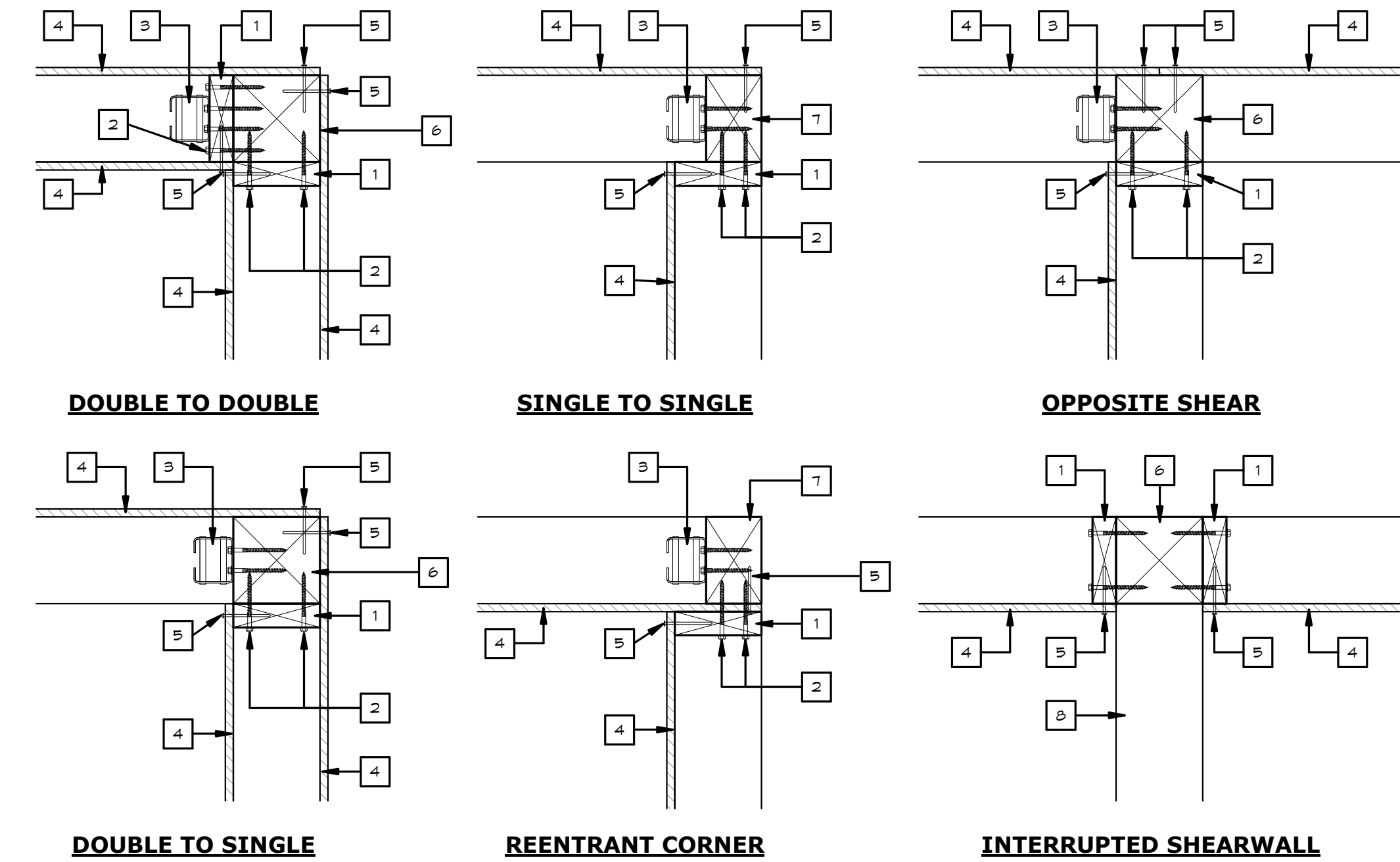
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drawn by MHC
reviewed by TWC
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project number 22018
drawing name

TYPICAL
DETAILS

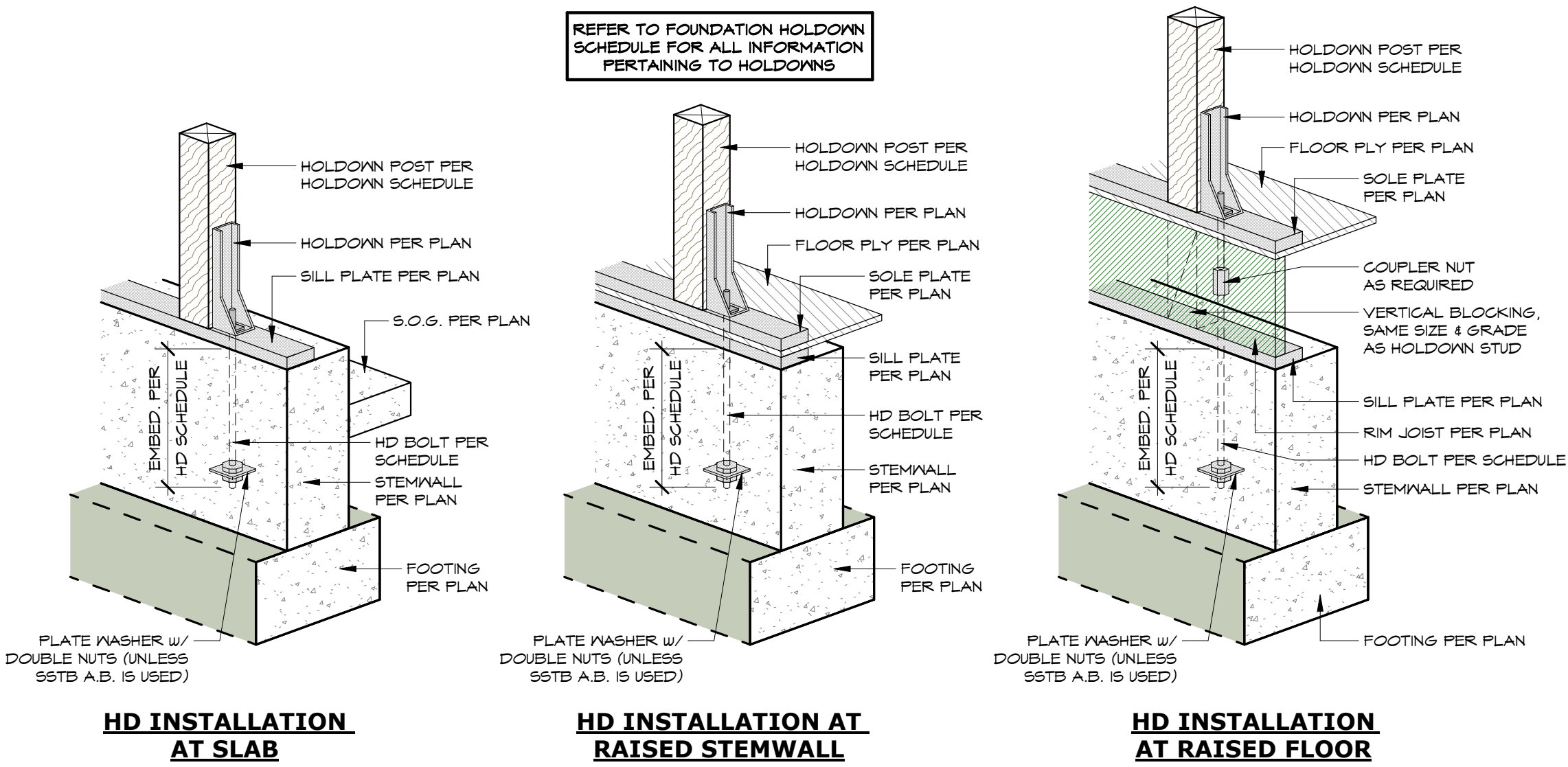
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S0.2



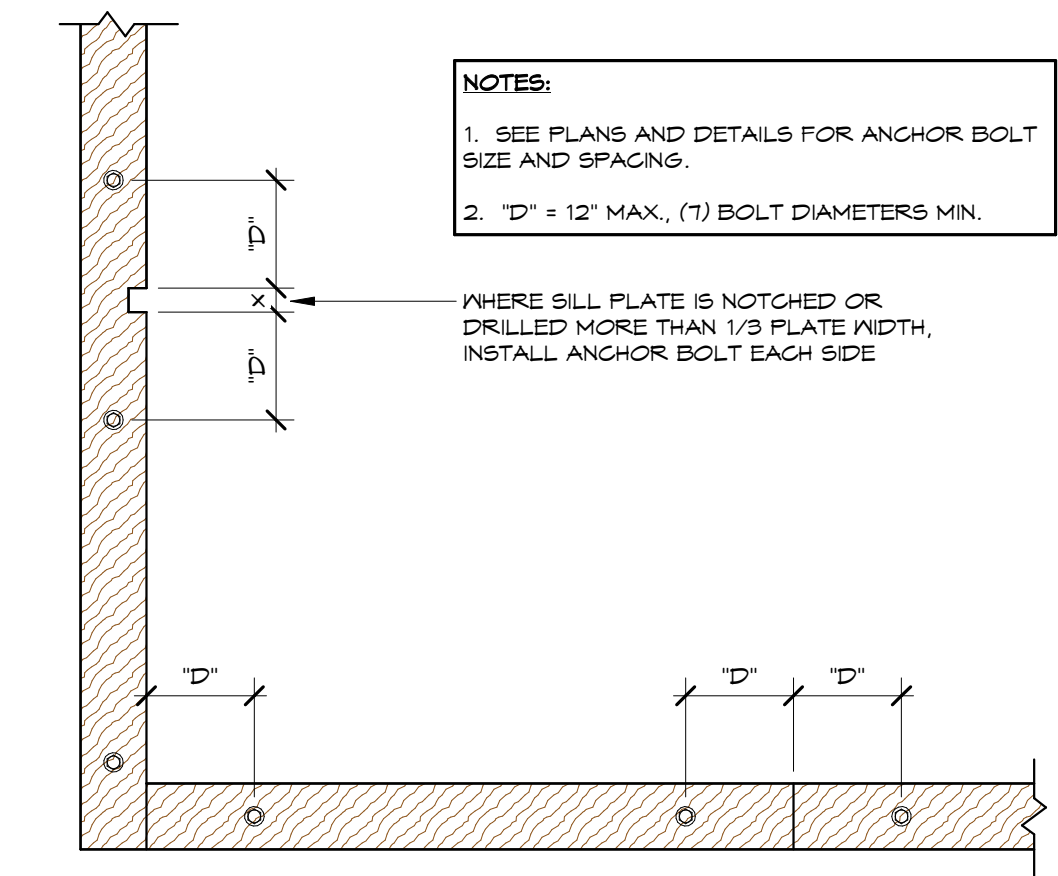
- LEGEND:**
- 2X OR 3X MEMBER RECEIVING SHEARWALL, EDGE NAIL (SEE SHEAR WALL SCHEDULE)
 - FOR TYPE "6" & "4" SHEARWALLS, USE (2) 16d NAILS @ 6" O.C.
- ALL OTHER SHEARWALL TYPES USE SDS 1/4"Ø x 4.5" SCREWS @ 4" O.C. STAGGERED ABOUT CENTERLINE OF STUD
 - HOLDOWN PER PLAN
 - SHEAR FLY PER PLAN
 - EDGE NAILING (SEE SHEAR WALL SCHEDULE)
 - SOLID FRAMING
 - HOLDOWN POSTS / STUDS PER PLAN (SEE SHEARWALL SCHEDULE)
 - GROSS WALL INTERRUPTING SHEARWALL

- NOTES:**
- SIMILAR SITUATIONS GET SIMILAR CONNECTIONS.
 - EVERY EXTERIOR / PERIMETER / LOAD BEARING CORNER SETS MINIMUM (3) STUDS OR (1) HOLDOWN POST & (1) STUD

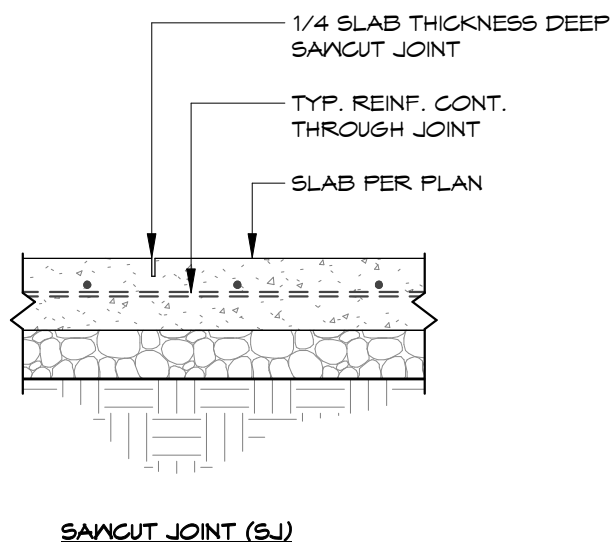


8 Typ. Foundation Holdown Detail
3/4" = 1'-0"

9 Typ. Holdown in Corner
1 1/2" = 1'-0"

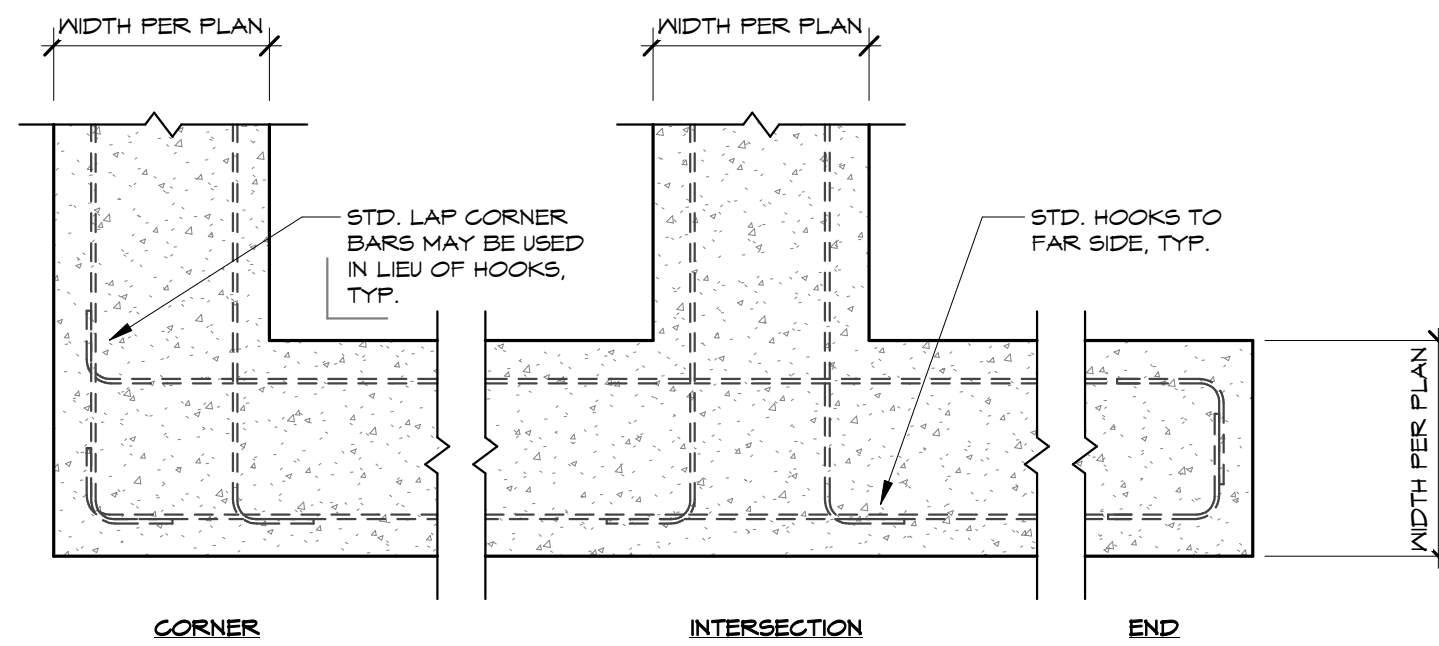


7 Typ. Sill Plate Detail
3/4" = 1'-0"



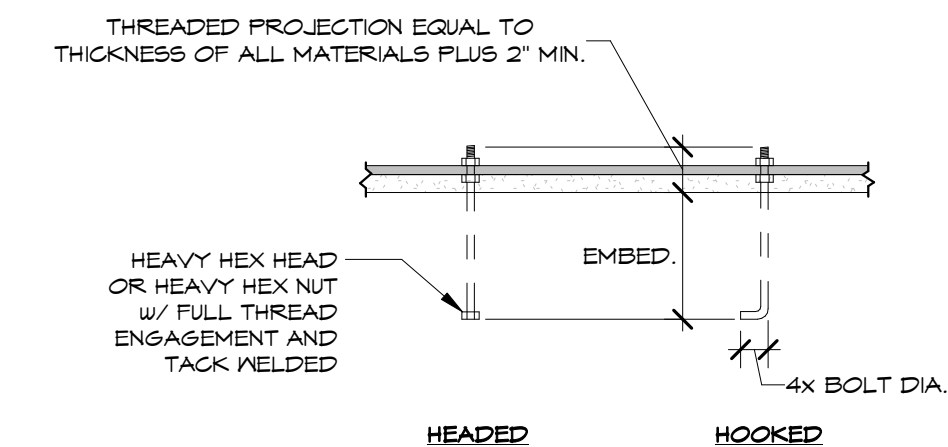
6 Typ. Slab Joints
3/4" = 1'-0"

- NOTES:**
- SLABS SHALL BE JOINTED EACH WAY IN ALTERNATING SANGUT JOINTS (SJ) AND DOVELEDED JOINTS (DJ OR PDJ) SPACED AT A MAXIMUM OF 2X SLAB THICKNESS, TYP. U.N.O. IE 2X 6" SLAB = 12" O.C. MAX. JOINT SPACING.
 - SLABS SHALL BE JOINTED SUCH THAT THE ASPECT RATIOS OF RESULTING INDIVIDUAL SLAB PANELS DOES NOT EXCEED 1.5:1, IE THE LONG SIDE OF AN INDIVIDUAL SLAB PANEL SHALL BE NO LONGER THAN 1.5 TIMES THE SHORT SIDE, TYP. U.N.O.
 - WHERE VAPOR BARRIERS OCCUR, THEY SHALL BE PLACED BELOW THE AGGREGATE BASE, TYP. U.N.O.

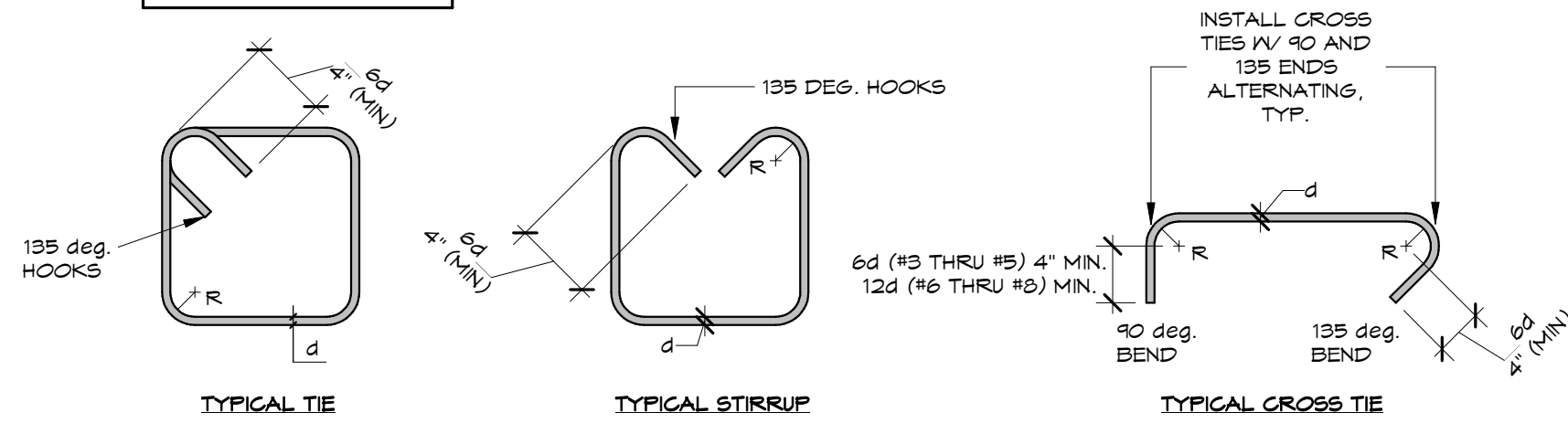


5 Typ. Continuous Footing Reinforcing
3/4" = 1'-0"

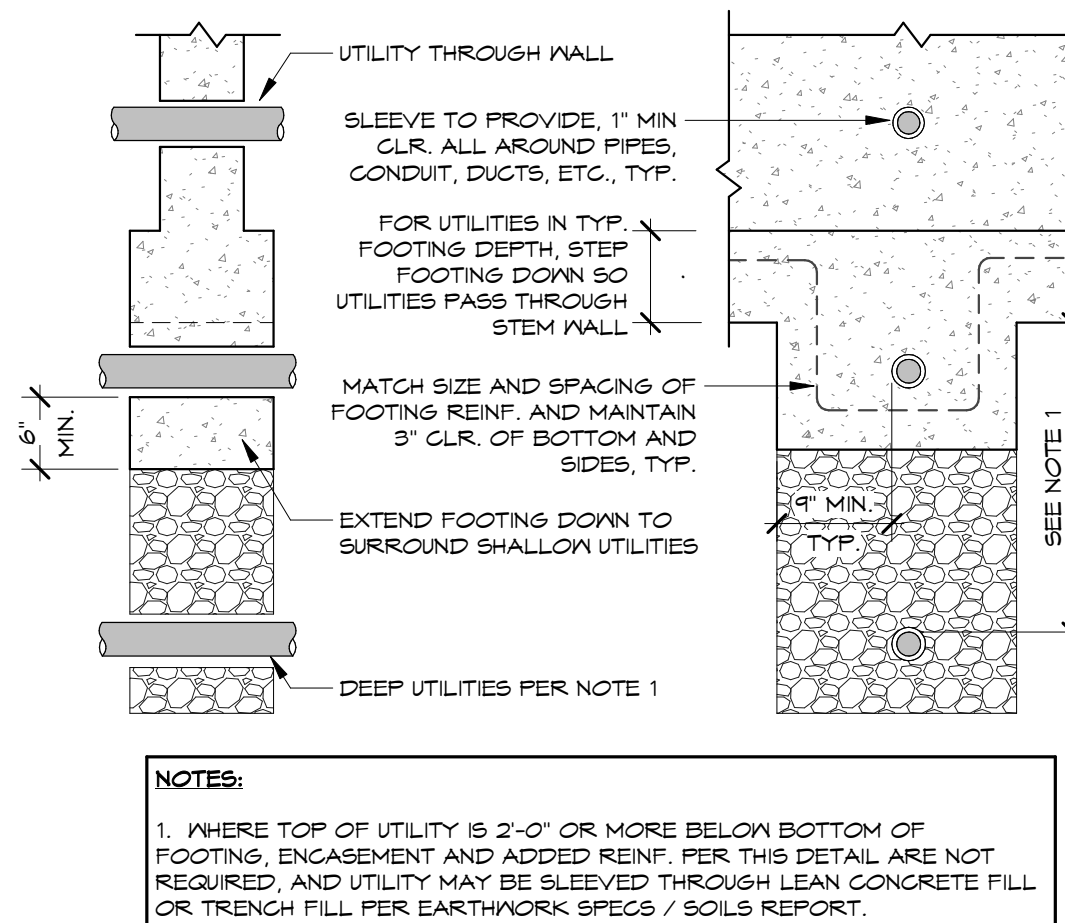
- NOTES:**
- SEE PLANS & DETAILS FOR REINFORCEMENT SIZE AND LOCATION.



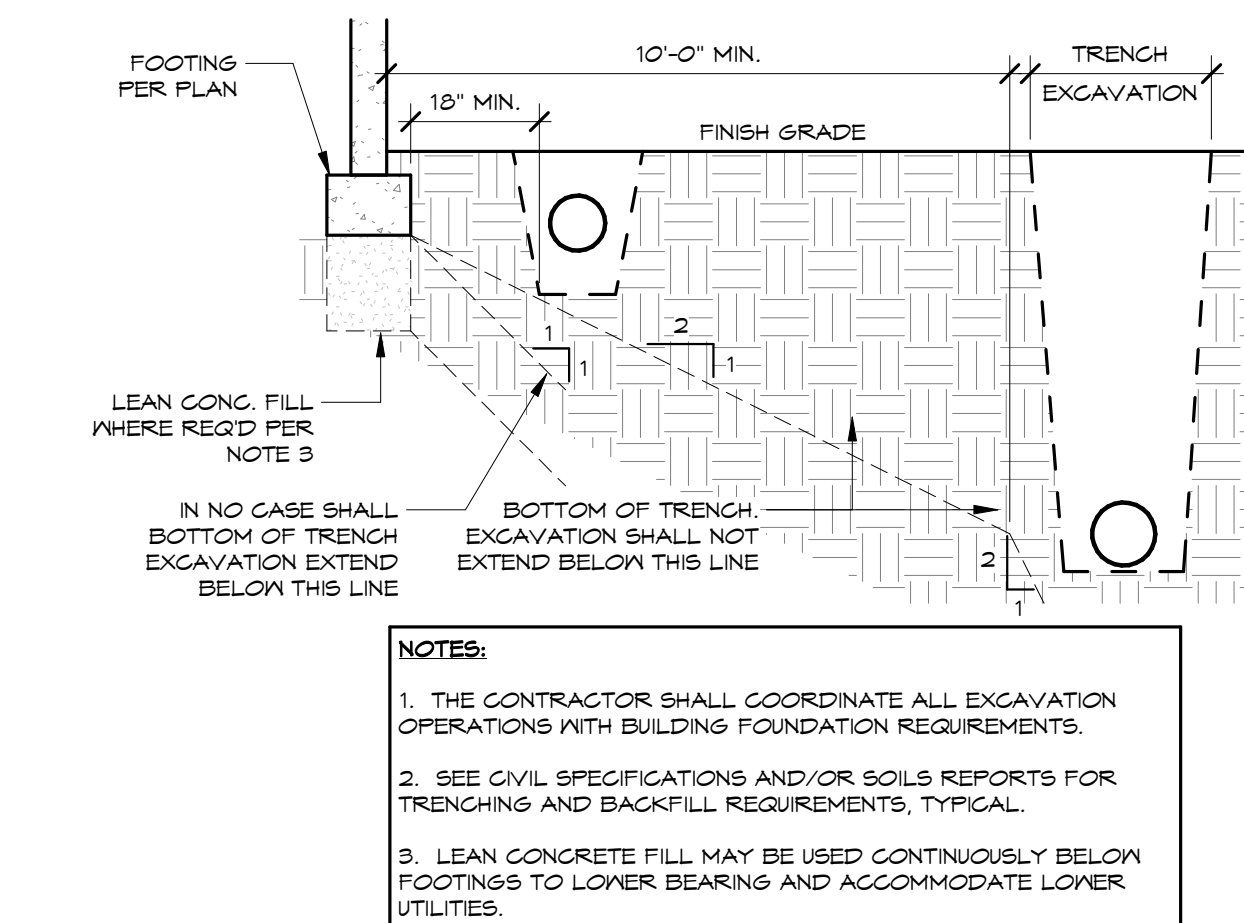
4 Typ. Anchor Bolts in Concrete
3/4" = 1'-0"



3 Typ. Rebar Hooks and Bends
3/4" = 1'-0"



2 Typ. Utility Through Stemwall or Continuous Footing
3/4" = 1'-0"



1 Typ. Trench Excavation Parallel to Footing
3/4" = 1'-0"

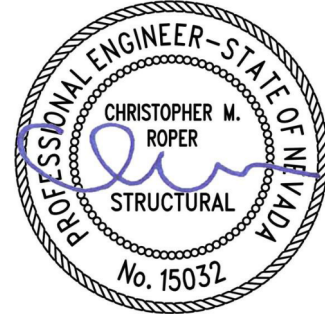
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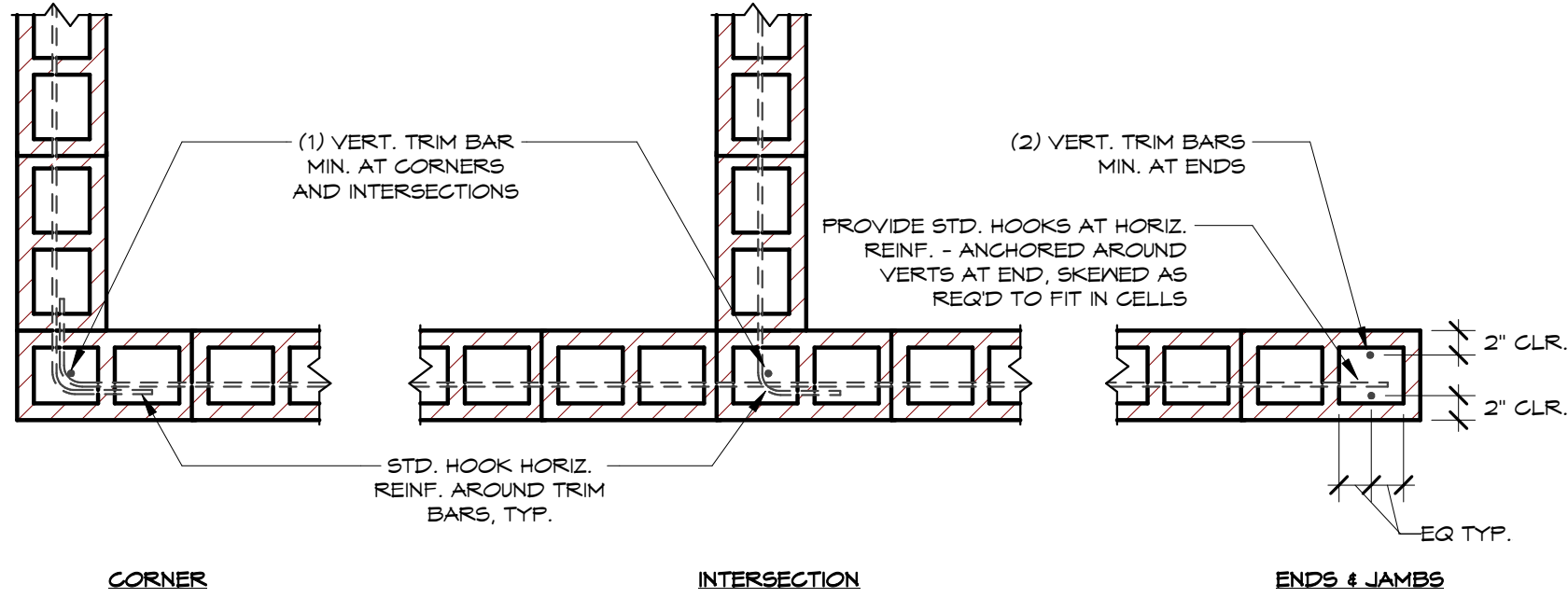
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TYPICAL
DETAILS

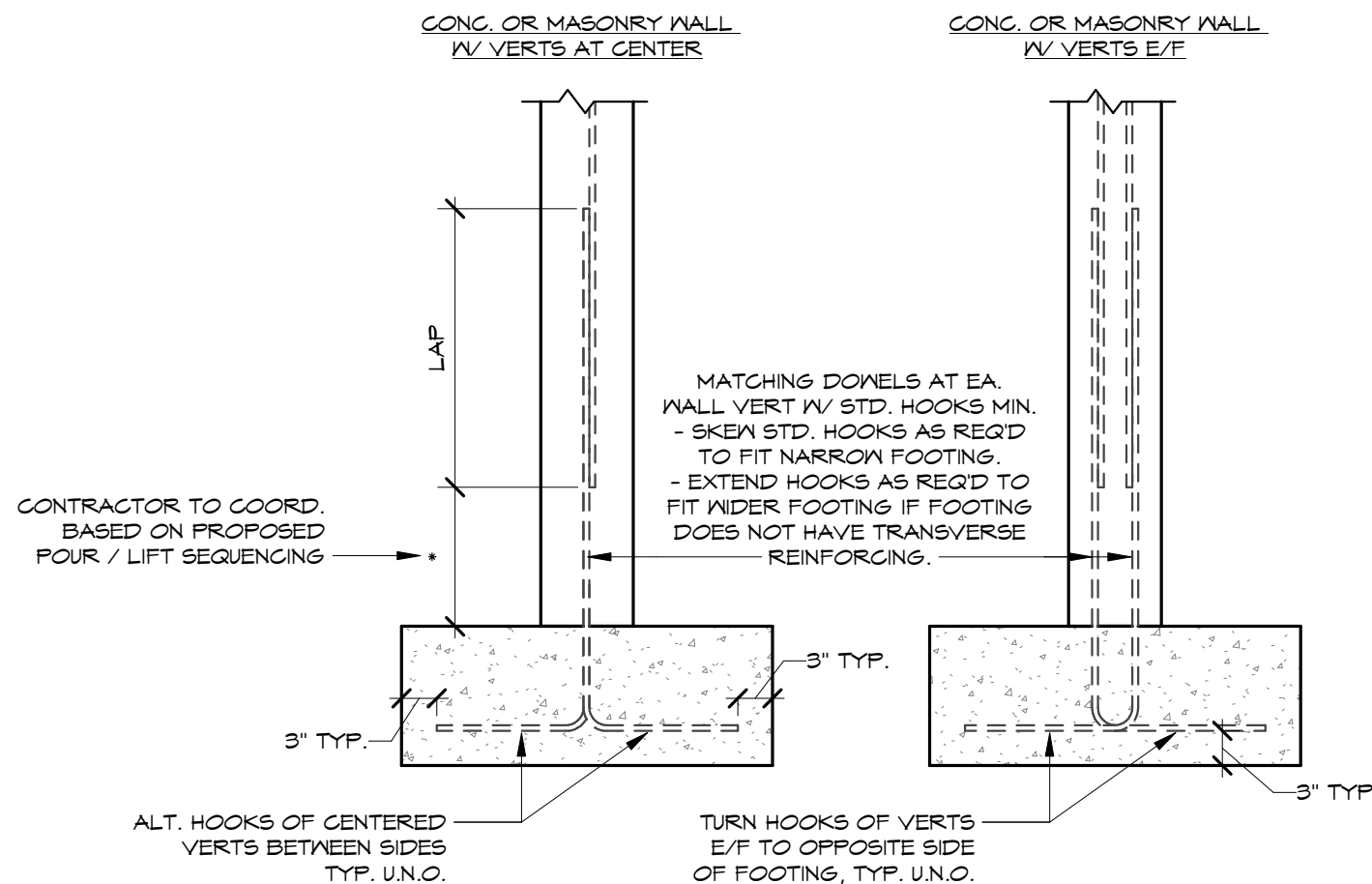
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S0.3

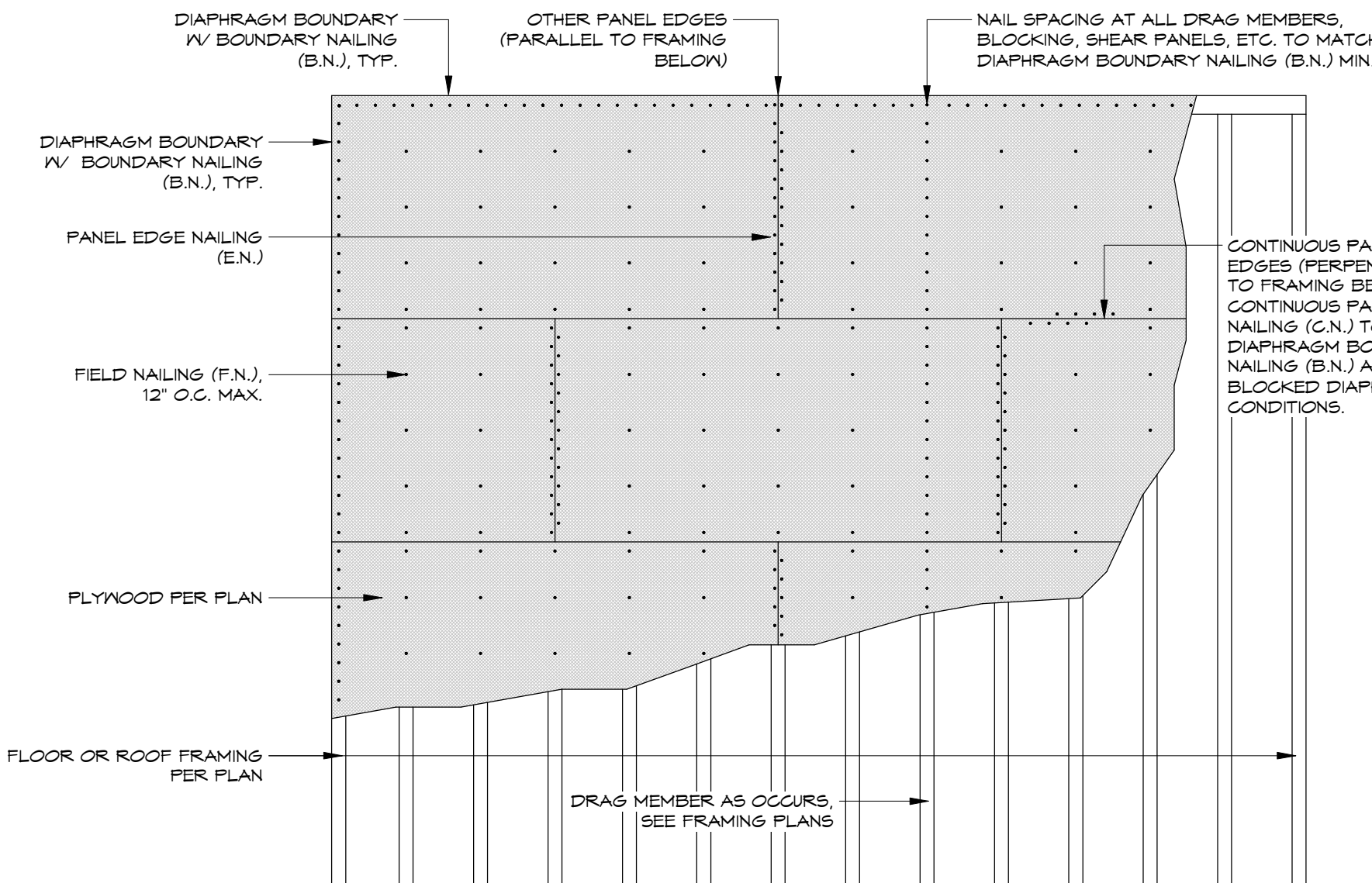
- NOTES:
1. VERT. TRIM BAR SIZE TO MATCH TYP. VERT. WALL REINF. SIZE U.N.O.
 2. CORNER BARS PROVIDING 4S DIA. LAP EA. DIRECTION MAY BE USED IN LIEU OF STD. HOOKED HORIZ. BARS. CORNER BAR SIZE TO MATCH TYP. HORIZ. WALL REINF. SIZE U.N.O.



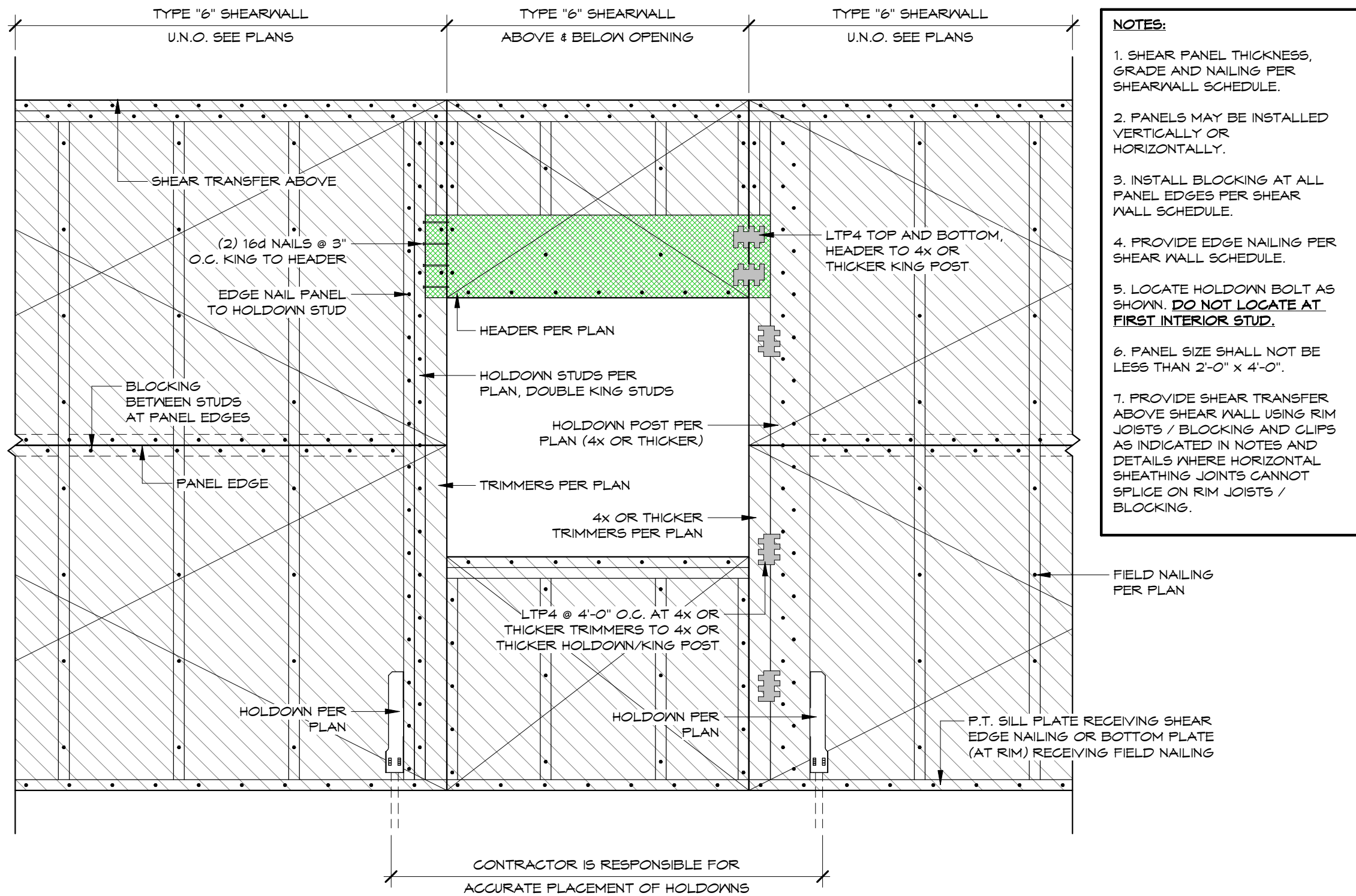
7 Typ. Masonry Wall Reinf. at Corners, Intersections and Ends
3/4" = 1'-0"



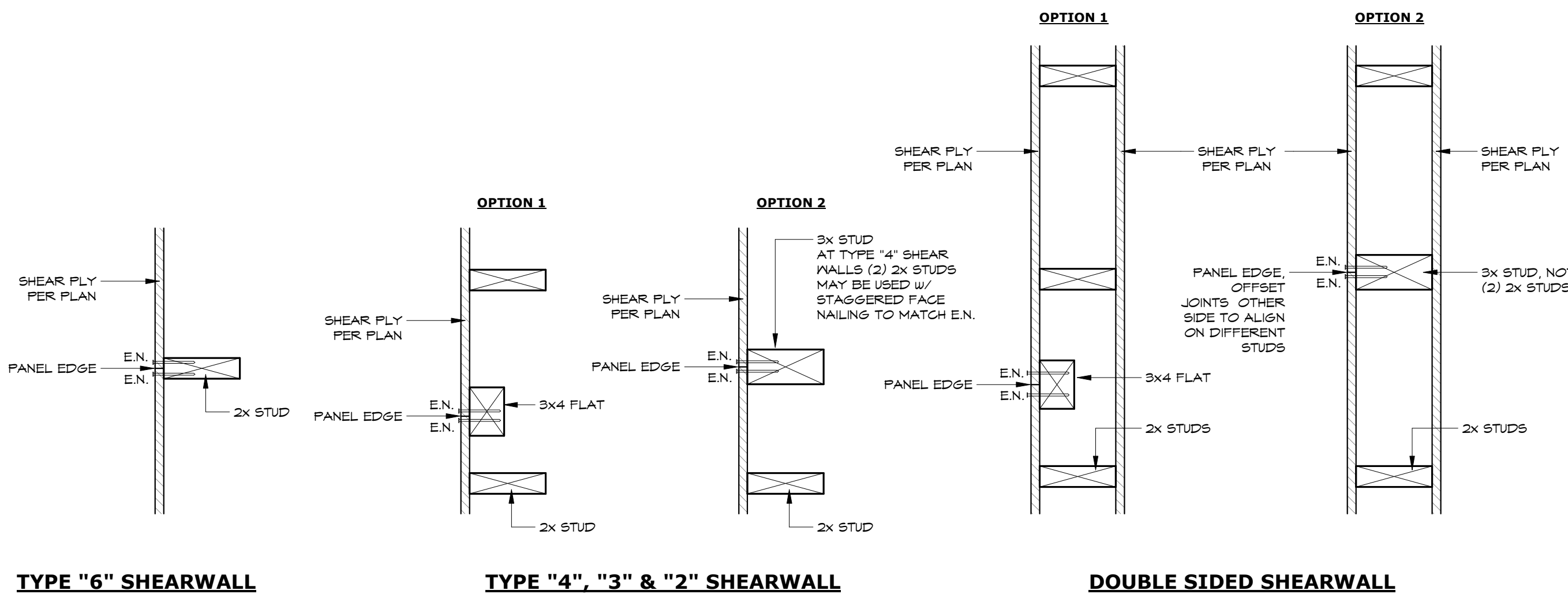
6 Typ. Wall Dowels at Cont. Footings
3/4" = 1'-0"



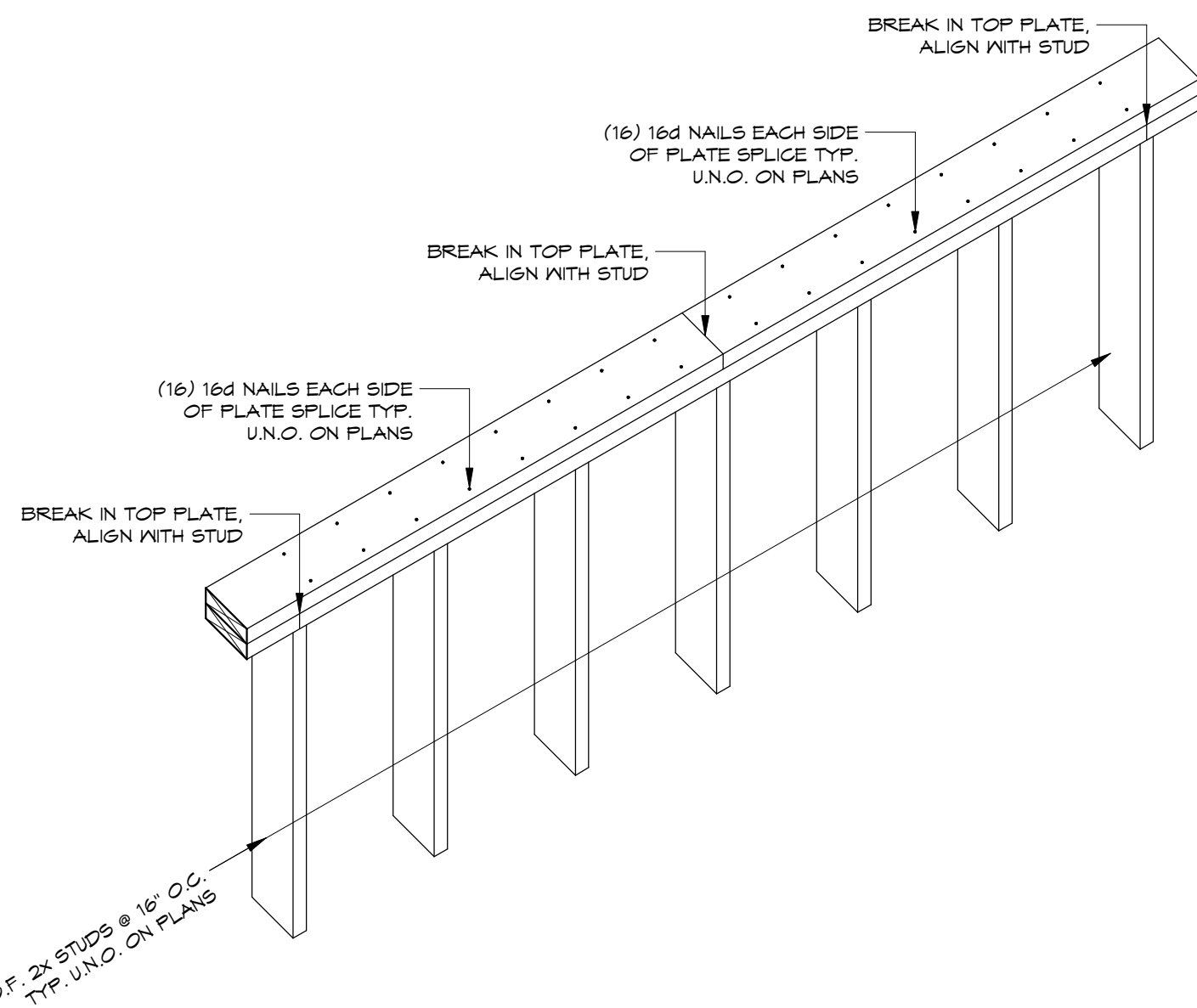
5 Typ. Horiz. Sheathing Layout and Fastening
3/4" = 1'-0"



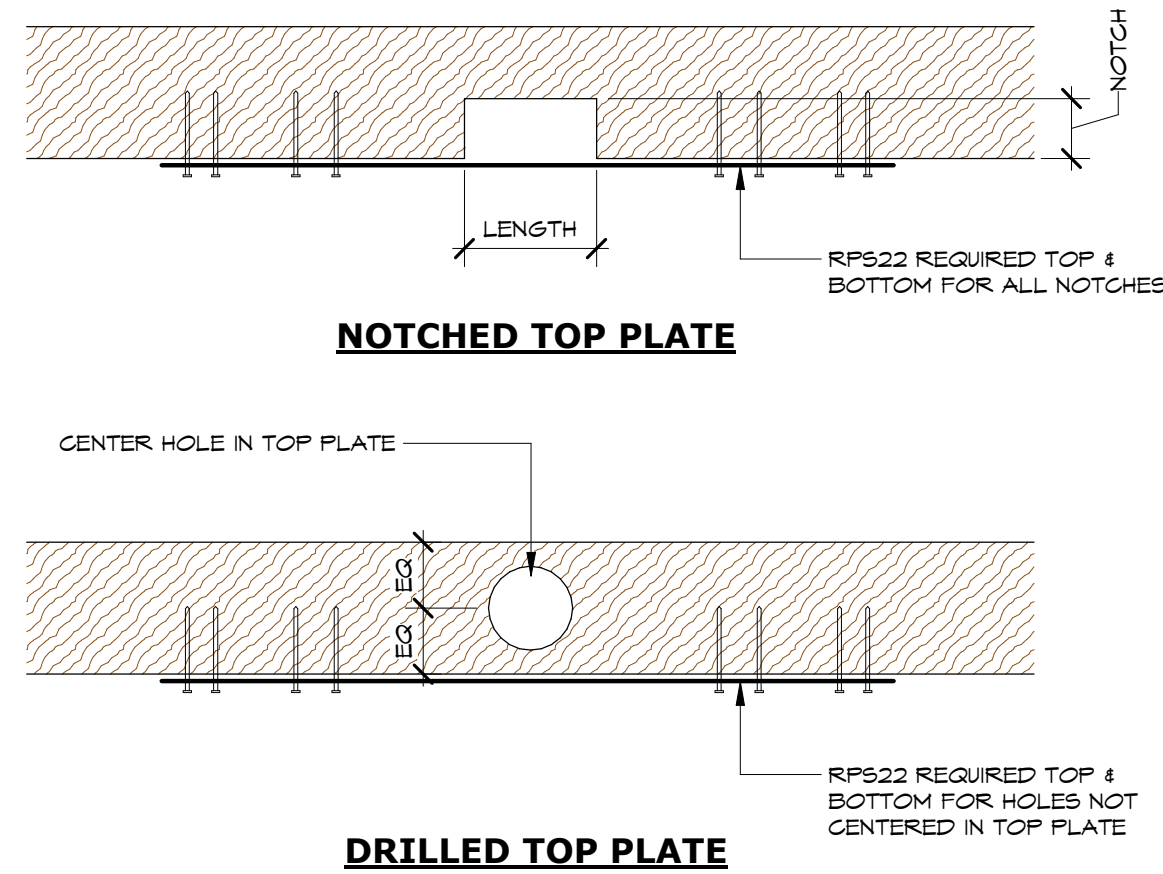
3 Typ. Shear Wall Detail
3/4" = 1'-0"



4 Typ. Shearwall Edge Members
1 1/2" = 1'-0"



2 Typ. Top Plate Splice Detail
3/4" = 1'-0"



WALL TYPE	MAX NOTCH	MAX DRILLED HOLE
2x4 BEARING	1' x 3 1/2"	1" DIA AT CL.
2x4 NON-BEARING	1 1/2" x 3 1/2"	1 1/2" DIA AT CL.
2x6 BEARING	2' x 5 1/2"	2" DIA AT CL.
2x6 NON-BEARING	2 1/2' x 5 1/2"	2 1/2" DIA AT CL.

1 Typ. Notches in Top Plate
1 1/2" = 1'-0"

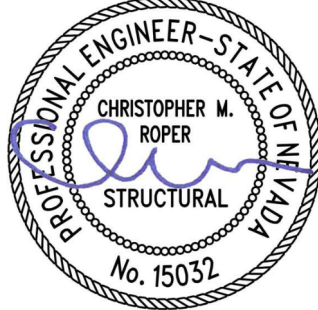
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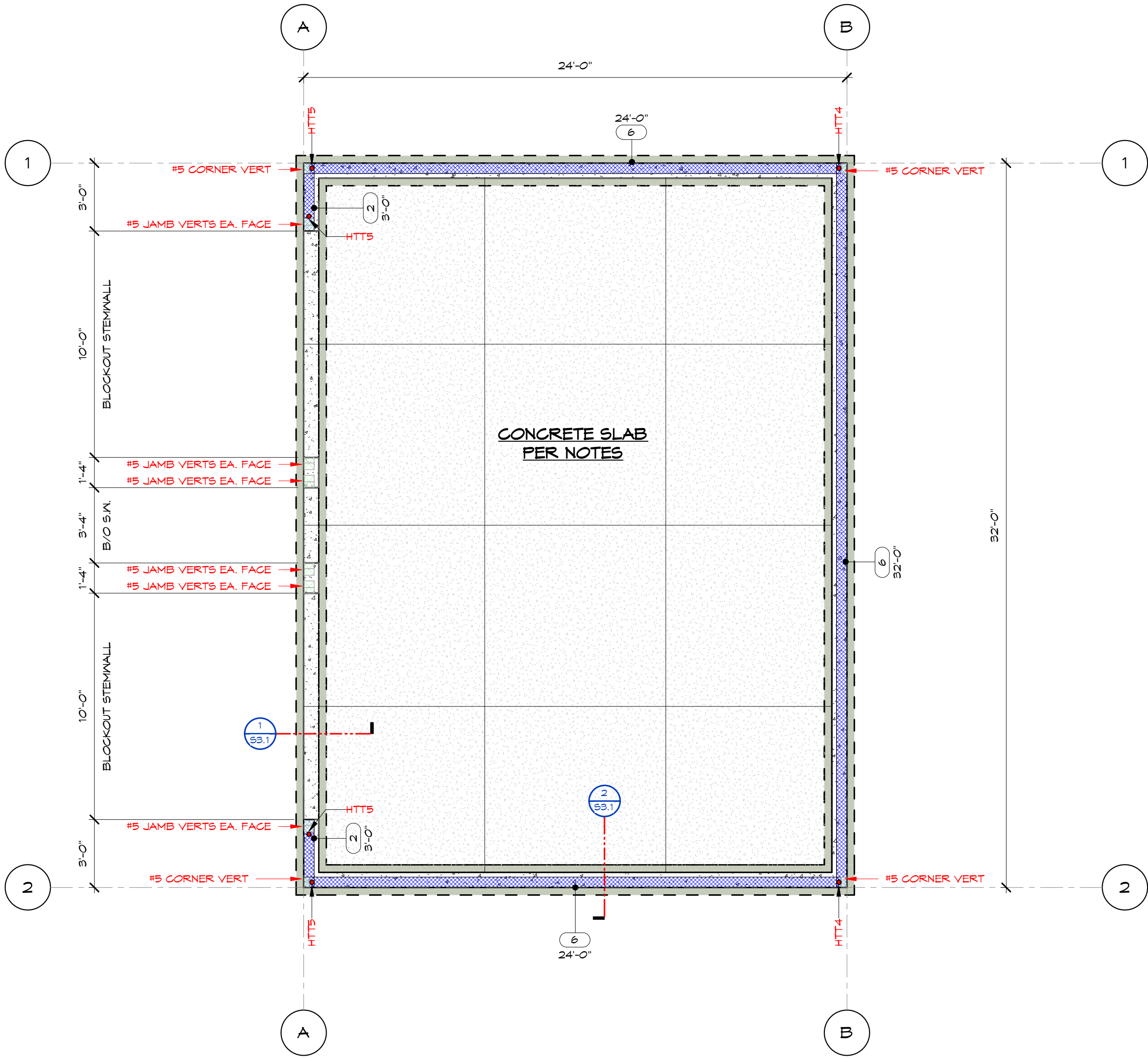
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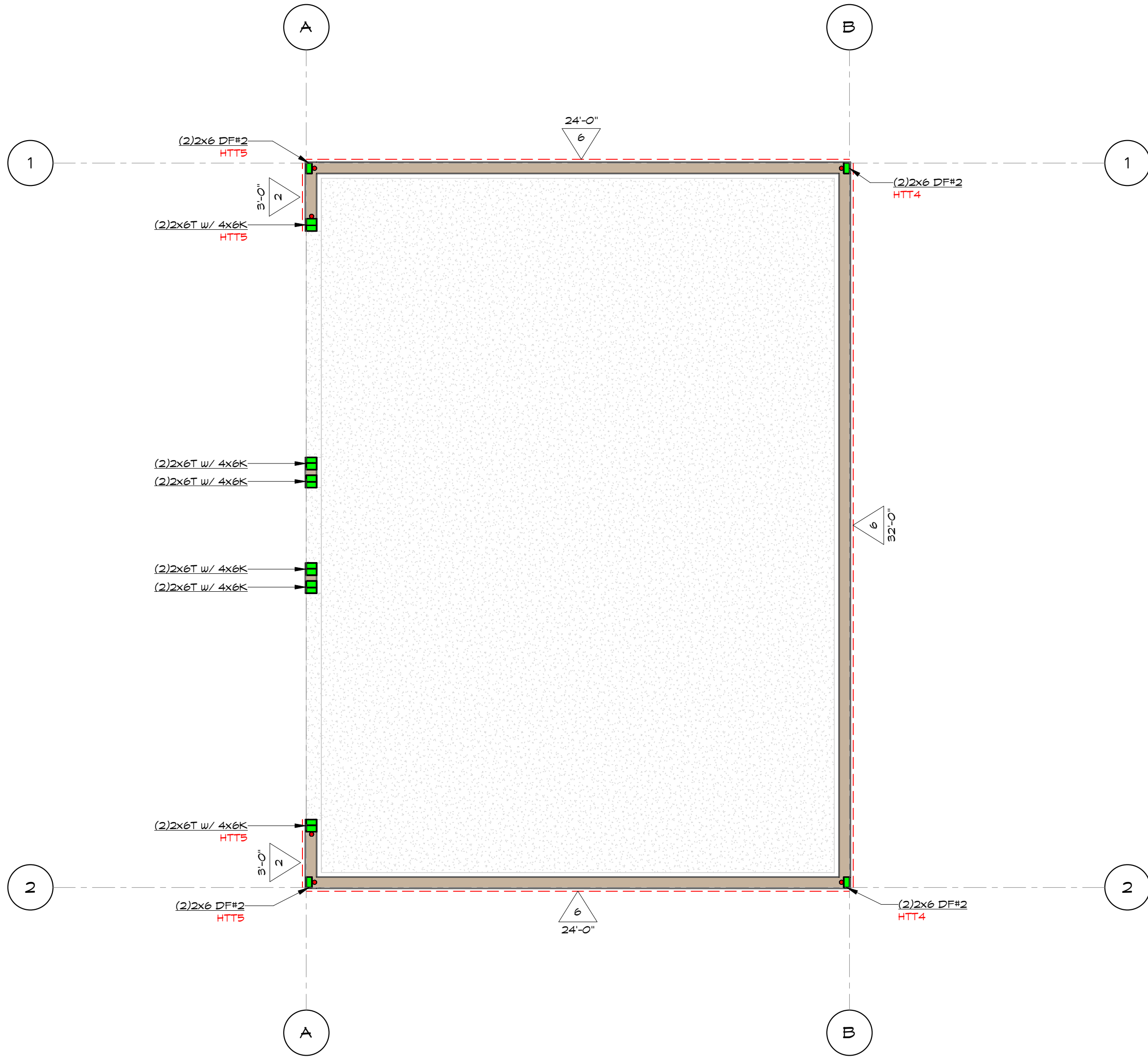
FOUNDATION
AND SHEAR
PLANS

sheet number

S1.1



1 FOUNDATION DIMENSION PLAN
1/4" = 1'-0"



2 MAIN LEVEL SHEAR & HOLDOWN PLAN
1/4" = 1'-0"

19/32" LP SMARTSIDE SHEAR WALL & SHEAR TRANSFER SCHEDULE						
SHEAR WALL SYMBOL	SHEAR PLY	BX PLATE REQ'D	EDGE NAIL (E.N.) SPACING	BX STUDS AT ADJOINING PANEL EDGES	SHEAR TRANSFER AT BLOCKING, RAFTER OR RIM JOIST	
					CLIP OPTION	SCREW OPTION
6	19/32"	NO	10d @ 6" O.C.	NO	A35 @ 16" O.C.	SDS 1/4"Φ x 6" @ 16" O.C.
4	19/32"	NO	10d @ 4" O.C.	NO	A35 @ 12" O.C.	SDS 1/4"Φ x 6" @ 12" O.C.
3	19/32"	NO	10d @ 3" O.C.	YES	L590 @ 16" O.C.	SDS 1/4"Φ x 6" @ 9" O.C.
2	19/32"	NO	10d @ 2" O.C.	YES	L590 @ 12" O.C.	SDS 1/4"Φ x 6" @ 2" O.C.
4/2	19/32" B.S.	YES	10d @ 4" O.C. BOTH SIDES	YES	H5A10 @ 12" O.C.	(2) SDS 1/4"Φ x 6" @ 12" O.C.
3/2	19/32" B.S.	YES	10d @ 3" O.C. BOTH SIDES	YES	H5A10 @ 8" O.C.	(2) SDS 1/4"Φ x 6" @ 8" O.C.
2/2	19/32" B.S.	YES	10d @ 2" O.C. BOTH SIDES	YES	H5A10 @ 6" O.C.	(2) SDS 1/4"Φ x 6" @ 6" O.C.
TYPICAL SHEAR WALL NOTES: - USE LP SMARTSIDE 19/32" PERFORMANCE CATEGORY PANELS NAILED AT SHIPLAP EDGES. SEE APA PRODUCT REPORT FR-1124 AND CG EVALUATION REPORT ESR-1301 FOR ADDITIONAL INFORMATION AND REQUIREMENTS. - USE 10d COMMON (0.149Φ x 3") EDGE NAILED AS INDICATED AND FIELD NAILED @ 12" O.C., U.N.O. - EDGE NAIL AT TOP PLATE, MID SILL, ALL POSTS, SOLE PLATES & ALL STUDS OR POSTS WITH HOLD DOWNS - PROVIDE BLOCKING AT HORIZONTAL EDGES OF SHEAR SHEATHING. - NAILS SHALL BE LOCATED 3/8" MIN. FROM EDGES OF PANELS AND FRAMING MEMBERS. NAILS WITH A ROW SHALL BE STAGGERED SO NAILS IN ADJACENT ROWS DO NOT ALIGN. - DOUBLE SHEAR WALLS TO HAVE SHEAR ON BOTH SIDES (OFFSET SHEATHING EDGES). - SEE SILL ANCHOR SCHEDULE FOR ADDITIONAL INFO AT SILL ANCHORS. - SEE TYPICAL DETAILS FOR ADDITIONAL INFO.						

TYPICAL FOUNDATION LEVEL NOTES:	
TYPICAL FOUNDATIONS: USE 16" WIDE, 12" DEEP CONT. FOOTINGS w/ (2) #4 CONT. AT BTM. SEE SECTIONS AND DETAILS FOR ADDITIONAL INFORMATION.	
TYPICAL CMU STEM WALLS: USE 8" CMU, GROUTED SOLID w/ #5 VERTS @ 32" O.C. MAX. AT WALL CENTER AND #4 HORIZ. @ 24" O.C. MAX. IN BOND BEAMS. USE #5 VERTS @ EA. CORNER AND #5 VERTS E/F @ DOOR JAMBS	
TYPICAL SLABS: USE 5" CONC. SLAB 1/4" #4 @ 18" O.C. EA. WAY AT 1'-1/2" CLEAR OF TOP, OVER 6" COMPACT TYPE 2 AGGREGATE BASE. SLABS SHALL BE JOINTED WHERE SHOWN ON PLANS AND/OR AS REQUIRED BY TYPICAL DETAILS. HARP OR SLOPE SLABS AS REQUIRED FOR DRAINAGE.	
TYPICAL SILL PLATE AND ANCHOR BOLTS: USE PRESSURE TREATED SILL PLATES TYP. USE 5/8"Φ x 10" ANCHOR BOLTS @ 48" O.C., TYP. U.N.O. (SEE ANCHOR BOLT SCHEDULE FOR ANCHOR BOLT SPACING REQUIREMENTS) USE (2) BOLTS MIN. EACH SECTION OF SILL, 12" MAX. FROM ENDS PROVIDE T MIN. EMBEDMENT ON ALL ANCHOR BOLTS TYP. USE 3"x3"x1/4" PLATE WASHERS AT ALL ANCHOR BOLTS TYP.	
TYPICAL WALL FRAMING: FRAME ALL WALLS USING 2x6 STUDS @ 16" O.C. MAX. KING AND TRIMMER STUDS SHALL MATCH TYPICAL STUD SIZE U.N.O. ALL STUD WALLS SHALL HAVE 2x MIN. BLOCKING THE SAME WIDTH AS STUDS, PREFERABLY AT MID-HEIGHT BUT NOT TO EXCEED 8" MAX. SPACING BETWEEN PLATES. SEE SHEAR, SILL AND HOLDOWN SCHEDULES FOR ADDITIONAL INFORMATION. ALL WALLS TO BE SHEATHED TO MATCH MINIMUM 6" O.C. SHEAR WALL REQUIREMENTS, EXCEPT HORIZONTAL BLOCKING MAY BE OMITTED AT WALLS NOT IDENTIFIED AS SHEAR WALLS.	

SILL ANCHOR SCHEDULE			
SYMBOL	SILL PLATE THICKNESS	SILL ANCHOR SIZE & SPACING	
6	2x PRESSURE TREATED	5/8"Φ x 10" A.B.'s @ 48" O.C. w/ 3"x3"x1/4" HDG. PL. WASHERS	
2	2x PRESSURE TREATED	5/8"Φ x 10" A.B.'s @ 16" O.C. w/ 3"x3"x1/4" HDG. PL. WASHERS	

FOUNDATION HOLDOWN SCHEDULE					
Model	HD Bolt C/L	Minimum Wood Member Thickness	Fasteners	Anchor Bolt Diameter	A.T.R. Embed. Single Pour
HTT4	0" - 1 5/16"	3"	(18) 16d x 2-1/2"	5/8"Φ	24"
HTT5	0" - 1 5/16"	3"	(26) 16d x 2-1/2"	5/8"Φ	24"

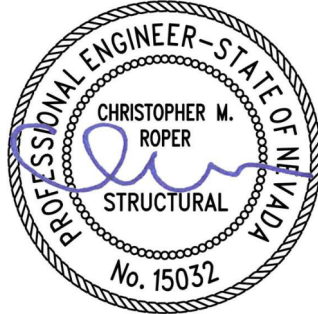
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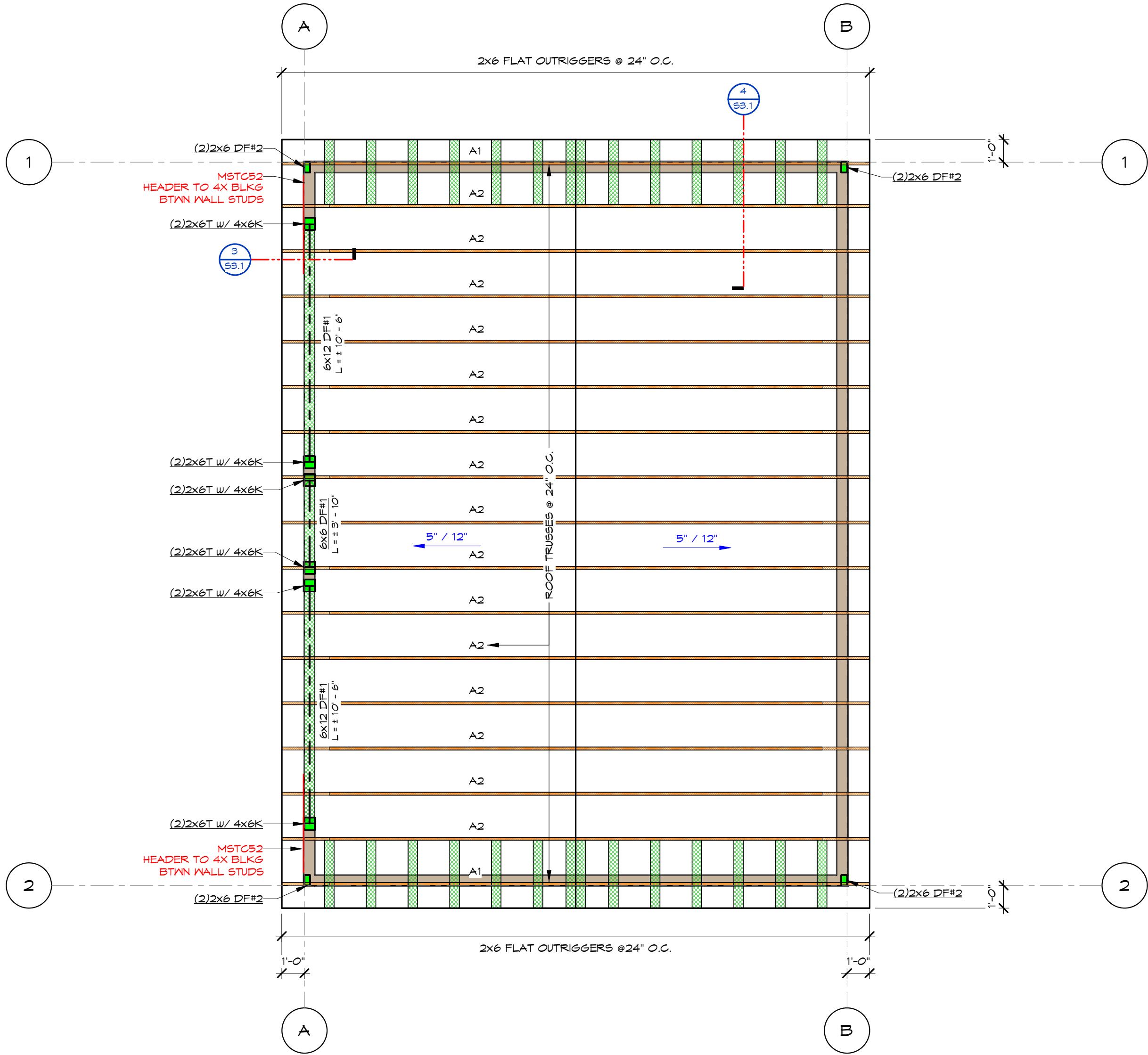
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drawing name	

MAIN ROOF
FRAMING
PLAN

sheet number

S2.1



1 MAIN ROOF FRAMING PLAN
1/4" = 1'-0"

TYPICAL ROOF LEVEL NOTES:

TYPICAL ROOF SHEATHING:

USE MIN. 1 1/2", APA RATED (40/20), EXPOSURE 1, PLYWOOD SHEATHING. NAIL 1/4" 10d x 2-1/2" MIN. COMMON DEFORMED SHANK NAILS AT 6" O.C. AT DIAPHRAGM BOUNDARIES & PANEL EDGES AND 12" O.C. MAX. IN THE FIELD. SHEATHING IS UNBLOCKED TYPICALLY. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.

TYPICAL ROOF FRAMING CONNECTORS:

USE H2.5A CLIPS AT EA. TRUSS TO DOUBLE TOP PLATES.
USE A38 CLIPS @ 48" O.C. TYPICAL AROUND BUILDING, I.E. FROM TOP PLATES TO BLOCKING BETWEEN TRUSSES OR FROM TOP PLATES TO GABLE END TRUSSES.

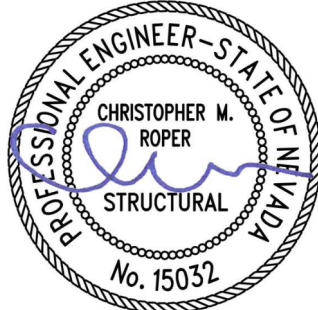
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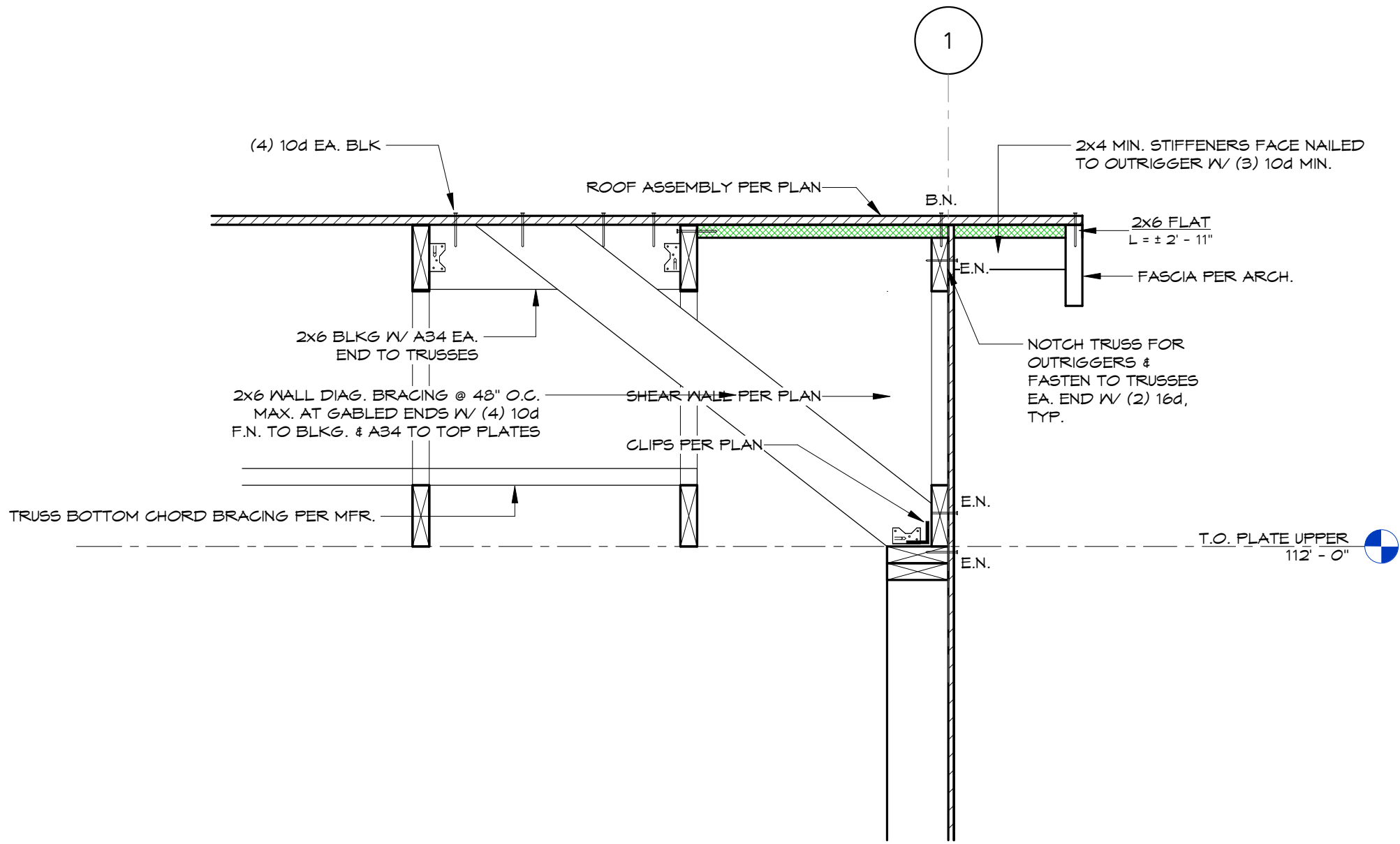
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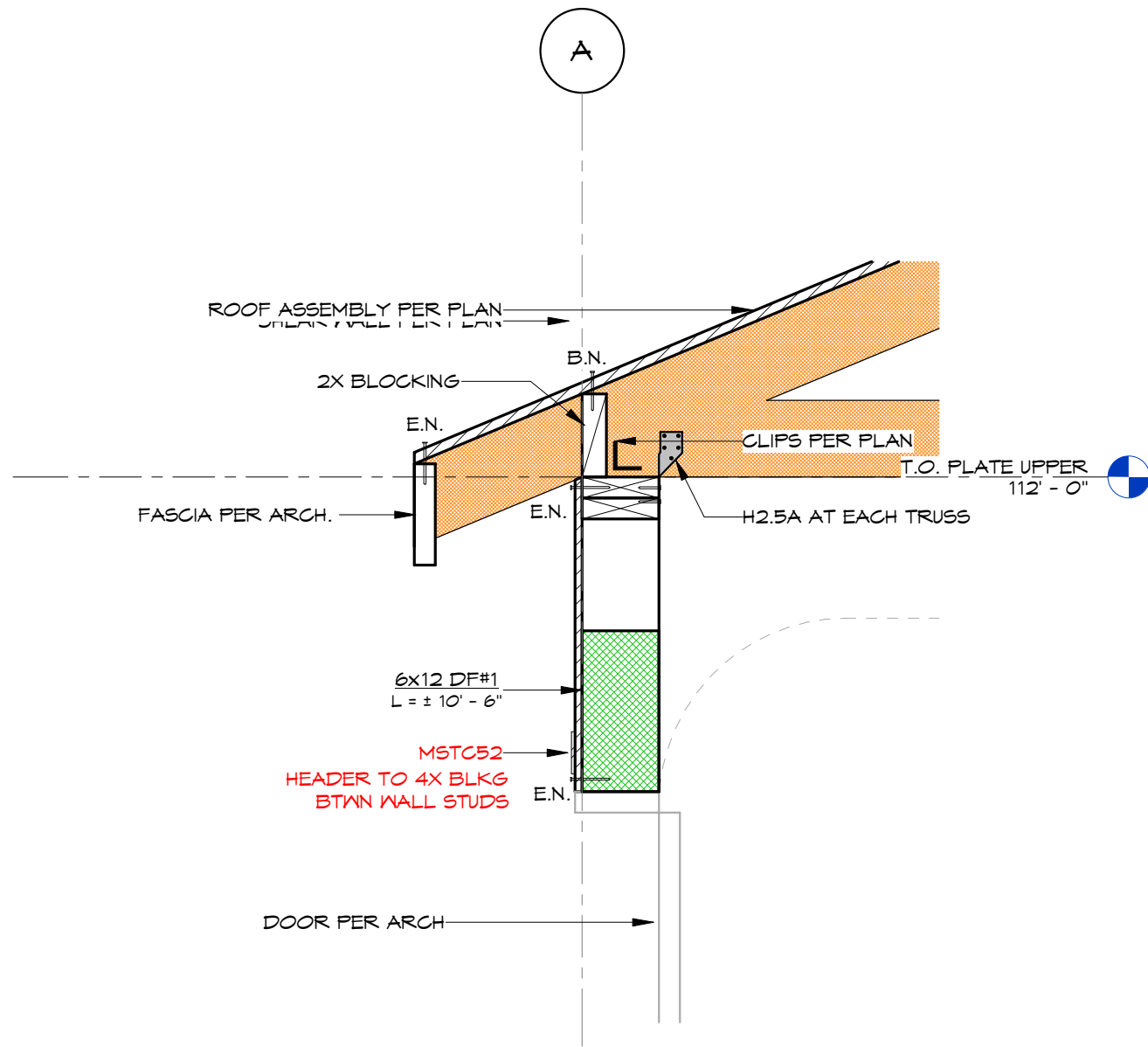
FOUNDATION
AND ROOF
FRAMING
DETAILS

sheet number

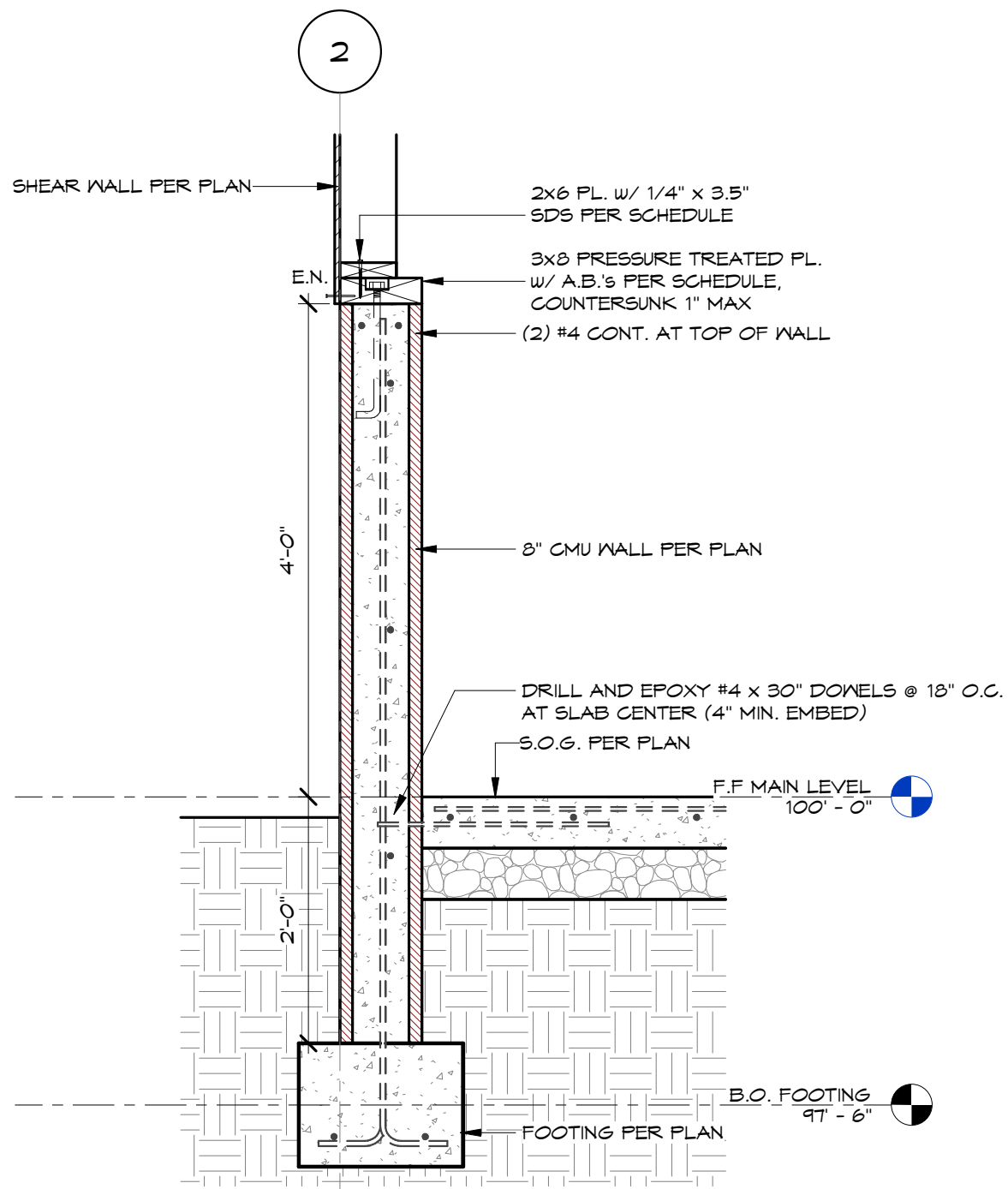
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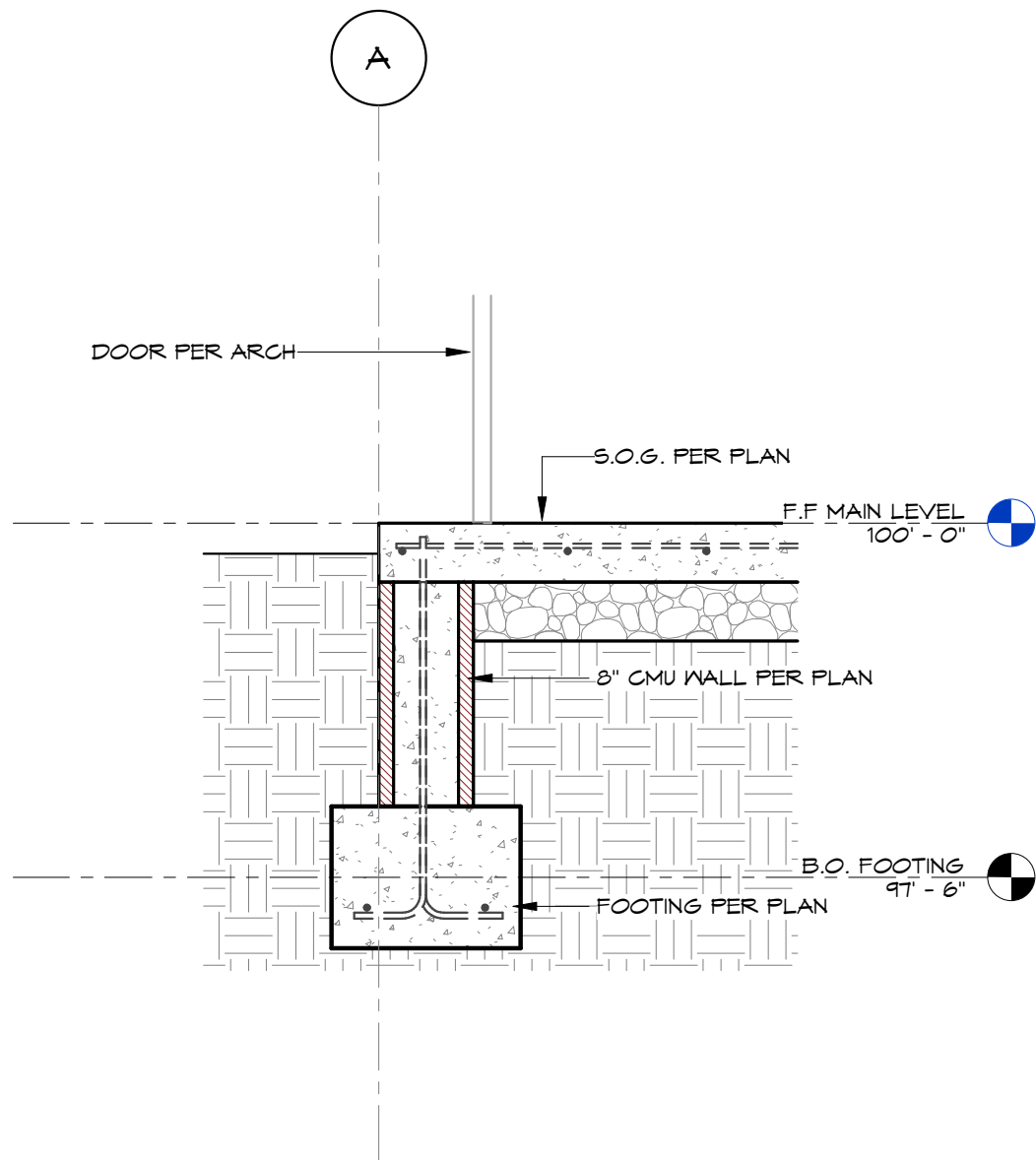
4 ROOF FRAMING DETAIL
1" = 1'-0"



3 ROOF FRAMING DETAIL
1" = 1'-0"






2 FOUNDATION DETAIL
3/4" = 1'-0"



1 FOUNDATION DETAIL
3/4" = 1'-0"

Electrical Legend

-  Electrical outlet
-  Electrical switch
-  8'-0" LED shop light

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FOR INFORMATION ONLY:
Douglas County Community
Development will seek a
separate electrical permit
package under a different
contract.

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Minden, Nevada 89423

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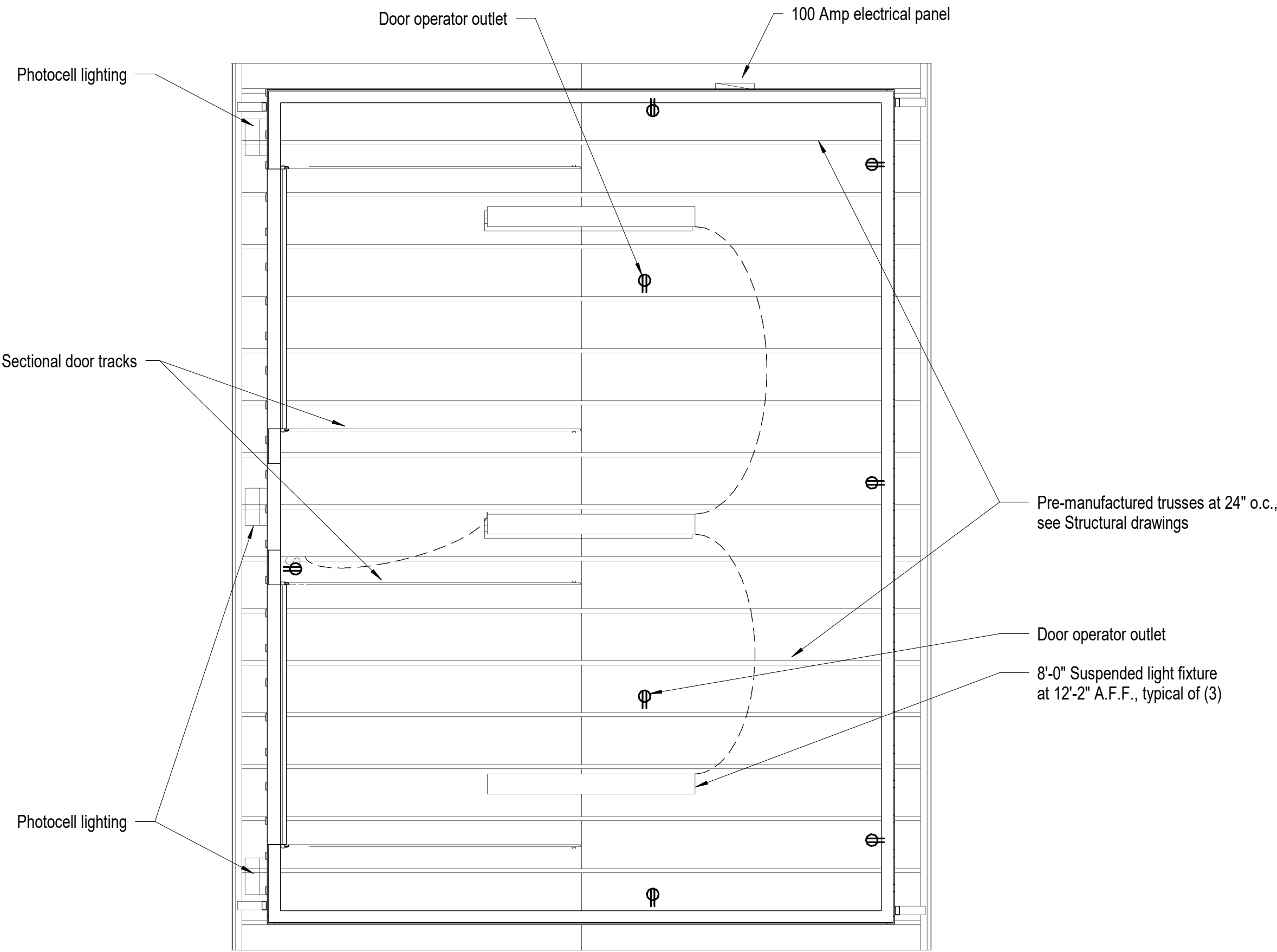
No.	Description	Date

drawn by	MLM
reviewed by	PAC
date	11/10/2022
project number	22018
drawing name	

Electrical Plan

sheet number

E101



North

